

City of Portsmouth, New Hampshire

Wetland Conditional Use Permit Application Checklist

This wetland conditional use permit application checklist is a tool designed to assist the applicant in the planning process and for preparing the application for Conservation Commission and Planning Board review. The checklist is required to be uploaded as part of your wetland conditional use permit application to ensure a full and complete application is submitted to the Planning and Sustainability Department and to the online portal. A pre-application conference with a member of the Planning and Sustainability Department is encouraged as additional project information may be required depending on the size and scope of the project. The applicant is cautioned that this checklist is only a guide and is not intended to be a complete list of all wetland conditional use permit requirements. Please refer to Article 10 of the City of Portsmouth Zoning Ordinance for full details.

Applicant Responsibilities: Applicable fees are due upon application submittal to the Planning Board (no fees are required for Conservation Commission submission). The application will be reviewed by Planning and Sustainability Department staff to determine completeness. Incomplete applications which do not provide required information for the evaluation of the proposed site development shall not be provided review by the Conservation Commission or Planning Board.

Name of Applicant: Rainboth Revocable Trust of 2010 Date Submitted: 2/20	/2025	
Application # (in City's online permitting): LU 25-13	_	
Site Address: 56 Ridges Court	Map: 207	Lot: 63, 68, 69

V	Required Items for Submittal	Item Location (e.g. Page or Plan Sheet/Note #)
'	Complete <u>application</u> form submitted via the City's web-based permitting program	completed on line
	All application documents, plans, supporting documentation, this checklist and other materials uploaded to the application form in OpenGov in digital Portable Document Format (PDF) . One hard copy of all plans and materials shall be submitted to the Planning and Sustainability Department by the published deadline.	completed, detailed on plan and in the narrative

Ø	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)
V	Basic property and wetland resource information. (10.1017.21)	completed on line
'	Additional information required for projects proposing greater than 250 square feet of permanent or temporary impacts. (10.1017.22)	setbacks, locations of wetlands, new and existing impervious areas are shown
V	Demonstrate impacts as they relate to the criteria for approval set forth in Section 10.1017.50 (or Section 10.1017.60 in the case of utility installation in a right-of-way). (10.1017.23)	completed, detailed on plan and in the narrative
'	Balance impervious surface impacts with removal and/or wetland buffer enhancement plan. (10.1017.24)	Reduction of impervious in buffer, see sheet C-2

Image: Control of the	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)
	Wetland buffer enhancement plan. (10.1017.25)	Buffer plantings and naturalization area is shown on Sheet C-2
	Living shoreline strategy provided for tidal wetland and/or tidal buffer impacts. (10.1017.26)	NA
'	Stormwater management must be in accordance with Best Management Practices including but not limited to: 1. New Hampshire Stormwater Manual, NHDES, current version. 2. Best Management Practices to Control Non-point Source Pollution: A Guide for Citizens and City Officials, NHDES, January 2004. (10.1018.10)	permeable driveway and infiltration area meets the intent of the NHDES BMP's.
'	Vegetated Buffer Strip slope of greater than or equal to 10%. (10.1018.22)	No portion of the 25-foot buffer is in excess of 10%
'	Removal or cutting of vegetation, use of fertilizers, pesticides and herbicides. (10.1018.23/10.1018.24/10.1018.25)	lawn will continue to be maintained. fertilizers, pesticides and herbicides wil
/	All new pavement within a wetland buffer shall be porous pavement. (10.1018.31)	See Sheet C-2
'	An application that proposes porous pavement in a wetland buffer shall include a pavement maintenance plan. (10.1018.32)	See O & M plan with drainage computations
/	Permanent wetland boundary markers shall be shown on the plan submitted with an application for a conditional use permit and shall be installed during project construction. (10.1018.40)	Lawn will continue to be maintained. Wetland buffer plaques
Ø	Requested Items for Submittal	Item Location (e.g. Page or Plan Sheet/Note #)
'	A narrative/letter addressed to the Conservation Commission Chair (if recommended to Planning Board then an additional narrative addressed to the Planning Board Chair at that time) describing the project and any proposed wetland and/or wetland buffer impacts. Please visit the WCUP instruction page for further application instructions.	in submission materials
'	If New Hampshire Department of Environmental Services (NHDES) Standard Dredge and Fill Permit is required for this work, please provide this permit application at the same time as your submission for a Wetland Conditional Use Permit.	Not required. No direct impacts are proposed

Applicant's Signature: _	Eric D. Weinrieb, PE	Date: 2-20-25



Civil Site Planning Environmental Engineering

133 Court Street Portsmouth, NH 03801-4413

February 20, 2025

Peter Britz, Planning and Sustainability Director City of Portsmouth Municipal Complex 1 Junkins Avenue Portsmouth, New Hampshire 03801

Re: Application for Conditional Use Permit Assessor's Map 207, Lots 63, 68 & 69 56 Ridges Court Altus Project No. 5639

Dear Peter,

On behalf of Annemarie (Annie) and Michael Rainboth, Trustees of the Rainboth Revocable Trust of 2010, Altus Engineering LLC (Altus) and the design team are pleased to submit a revised n application for a Conditional Use Permit and wish to be heard at the March 12th Conservation Commission meeting. Annie and Michael own the property located at 56 Ridges Court. They currently live a few houses away on the corner of Ridges Court and New Castle Avenue. They intend to renovate and expand the existing home.

The entire neighborhood was constructed prior to City wetland buffer regulations. Portions of the lot are within the NHDES 100-foot tidal buffer and the 250-foot Shoreland Buffer. No improvements are proposed within 100 feet of the highest observable tide line. A permit from the NHDES Shoreland program will be required.

The existing garage and shed will be razed. A garage addition with living space above is proposed along with a new shed. The structures will be further from the resource area than the existing buildings. Stormwater management improvements are proposed to enhance the wetland buffer.

At the February 12th Conservation Commission meeting, the Commission voted to continue the application to the March hearing.

Based on the comments of the Commission, Altus working with the Rainboth's have revised the application package.

- The shed has been reduced in size and moved further from the wetlands.
- The deck has been reduced in size and moved further from the wetlands.

Tel: (603) 433-2335 E-mail: Altus@altus-eng.com

- The portion of the driveway within the wetland buffer will be constructed using permeable pavers or porous pavement.
- We are no longer proposing to consolidate the three parcels as the Chair expressed that consolidating the lots had no positive impact on the application.
- There will be a reduction in impervious on the entire site as well as within the 100-foot wetland buffer.
- An 1,100 SF area of the wetland buffer is designated to be naturalized.
- Additional native plantings are proposed in the buffer.

Enclosed for the Commission's consideration please find the following:

- Letter of Authorization
- Conditional Use Permit Narrative (revised)
- Wetland Buffer Function and Values Assessment (Noel)
- Drainage computations
- Stormwater O & M manual (revised)
- Project Site Plans (revised)

The drainage computations have not been revised as the conversion of the driveway to a permeable surface has no impact on the calculations as the driveway was already designed to infiltrate. The deck was designed to infiltrate as well, thus the reduction has no impact. The reduced shed footprint will create a slight reduction in the rate and volume of runoff.

Please feel free to call or email me directly should you have any questions or need any additional information.

Sincerely,

ALTUS ENGINEERING, LLC

Enclosures

eCopy: Annie and Mike Rainboth

Joseph Noel, Wetlands Scientist

Amy Dutton

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Letter of Authorization

I, Annemarie Rainboth and Michael Rainboth, Trustees of The Rainboth Revocable Trust of 2010, owner of the property located at 56 Ridges Court, Portsmouth, NH, hereby authorize Altus Engineering, LLC of Portsmouth, NH to represent us as the Owner and Applicant in all matters concerning the engineering and related permitting on Portsmouth Tax Map 207, Lots 63, 68, and 69, Portsmouth, New Hampshire. This authorization shall include any signatures required for Federal, State and Municipal permit applications.

Signature Annemarie Rainboth Date	
Sophia Howe Josephia Howe Witness Print Name Date	118/2025
Mi-Lame 1/8/2025 Signature Michael Rainboth Date	
Sophia Howe Sophie Howe Witness Print Name Date	1/8/2025



Civil Site Planning Environmental Engineering

133 Court Street Portsmouth, NH 03801-4413

CONDITIONAL USE PERMIT APPLICATION 56 RIDGES COURT NARRATIVE February 20, 2025

On behalf of the Applicant, Annemarie (Annie) and Michael Rainboth and the Rainboth Revocable Trust of 2010, Altus Engineering, LLC (Altus) respectfully submits a Wetlands Conditional Use Permit application for the expansion of a single-family residence at 56 Ridges Court. Annie and Mike propose to retain, renovate, and expand the 100-year-old home and raze the existing outbuildings.

The home is sited on Tax Map 207, Lot 63. Two additional vacant parcels, Tax Map 207 Lots 68 and 69, are contiguous to the Rainboth's future home. The vacant lots appear to have frontage on a paper street. For the basis of the computations, all three lots being contiguous are included.

The southeast corner of the parcel lies within the 100-foot tidal buffer. We are proposing to avoid impacting the tidal buffer. The southeastern portion of the lot is a freshwater wetland. The 100-foot buffer from the wetland encompasses a significant portion of the lot, making redevelopment of any sort nearly impossible without a Conditional Use Permit. The majority of the on-site wetland is maintained as lawn.

The house was constructed prior to City wetland buffer regulations and before most zoning ordinances were enacted. Based on the topography adjacent to the existing driveway, it appears that portions of the lot were regraded and filled.

The existing garage is over 80-feet from Ridges Court requiring a long driveway and turnaround area. The expanded home will be sited close to the street, reducing the driveway substantially.

The built infrastructure will be sited further from the resource area than the current buildings and pavement. Stormwater management treatment will be provided where none currently exists. The Rainboth's are good stewards of the land and want to keep the back yard lawn as a maintained lawn. They are committed to avoiding the use of herbicides, pesticides, and fertilizers in the wetlands and across their whole property.

In accordance with Article 10 Environmental Protection Standards Section 10.1010 Wetland Protect, the redevelopment will require a Conditional Use Permit from the Planning Board. The project does not require any additional relief from the City of Portsmouth Zoning Ordinance.

Tel: (603) 433-2335 E-mail: Altus@altus-eng.com

Per Section 10.1017.50 for criteria for approval of a Conditional Use Permit, Altus offers the following:

(1) The land is reasonably suited to the use, activity, or alteration.

The property is within the SRB Zoning District, a residential zone. All of the abutting properties are residential. The parcel has been used as a single-family residence for nearly 100-years and will continue to do so. The minimum lot size in the zoning district is 15,000 SF. The existing lot is 20,585 SF in area. Consolidated, the lot will exceed 30,000 SF, enough land to subdivide the land into two parcels.

The existing home is served by municipal water supply and is connected to the municipal sewage collection system. Commercial use of the property is not allowed. As such, the only viable use of the property is a single-family residence.

(2) There is no alternative location outside the wetland buffer that is feasible and reasonable for the proposed use, activity, or alteration.

Consolidated, the 30,962 SF parcel exceeds the minimum lot size for the zoning district. Only 3,550 SF of the lot is not within the wetland buffer and the majority of that area is within the front and side yard setbacks which are not buildable by right or are sited in the rear of the lot requiring a long access drive across the buffer for access. Only 725 SF of the lot exclusive of the existing building is viable for development without obtaining a variance or conditional use permit.

Thus, there is very little viable building envelope that meets both the zoning setbacks and is outside the wetland buffer area. The development proposed is sited as far from the resource area as reasonably possible. The Rainboth's are taking advantage of retaining the existing home and expanding it. Due to the layout of the existing structure and the desire to have a two-bay garage, the addition needs to be attached to the rear of the home and then will extend south to provide access to the garage.

(3) There will be no adverse impact on the wetland functional values of the site or surrounding properties;

The majority of the on-site wetland system is maintained as lawn and has been for several decades.

Along the property line, the wetland transitions to a natural environment with scrub growth. The wetland/lawn encompasses 6,100 SF. No impacts or changes are proposed to this area. The lawn functions as a stormwater filter, natural

detention, and moderates the velocity of runoff discharging from the neighborhood.

Currently upgradient of the wetland is the house, garage/shed, and large paved driveway. The existing expansive driveway is within 32-feet of the wetlands. The building and pavement will be moved further from the wetland. The proposed deck, which will be permeable beneath, will be 51.5 feet from the wetland. Drip edges will be installed on the west side of the building to promote infiltration, reduce the rate of runoff, and provide treatment. Runoff from the east and north side of the building will be captured in gutters and will be directed to the infiltration system beneath the deck. The portion of the driveway within the wetland buffer will be constructed using permeable materials, either pavement or pavers. Runoff from the impervious portion of the new driveway will be routed across the lawn through a swale that will treat, reduce the velocity, and reduce runoff temperature before discharge into the wetland.

Stormwater quantity will be enhanced and volume and peak rate of runoff discharging from the site will be reduced.

The site effective impervious area will be slightly reduced in both the wetland buffer and the entire lot, as we are taking advantage of the area beneath the deck to provide groundwater recharge and infiltration.

(4) Alteration of the natural vegetative state or managed woodland will occur only to the extent necessary to achieve construction goals; and

> The entire redevelopment project will be within areas that have previously been altered. Five trees and shrubs within the buffer will be removed. To offset the removal, 5 new wetland tolerant trees will be planted. Additionally, a ±1,100 SF portion of the existing lawn in the wetland buffer will be allowed to naturalize and will no longer be maintained as lawn. 6-high bush blueberry plants will be installed at the end of the swale adjacent to the wetlands.

(5) The proposal is the alternative with the least adverse impact to areas and environments under the jurisdiction of this Section.

> The proposed project will impact approximately 8,800 SF of land area within the wetland buffer. All of the impacts will be within previously developed areas that are either lawn, building, or driveway. The design approach avoids impacting natural areas. The house addition is placed as close to the front lot line as reasonably possible and remain compliant with the zoning ordinance and provide natural flow of the interior of the existing house to the addition and garage, while

providing adequate space for parking in the driveway for visitors as Ridges Court is narrow and has limited opportunities for street parking.

(6) Any area within the vegetated buffer strip will be returned to a natural state to the extent feasible.

In lieu of restoring the wetland to the natural state and providing a natural buffer, the Rainboth's are allowing a portion of the lot to naturalize, and adding native plantings.

5639-rev 1 cup narrative.docx



JOSEPH W. NOEL P.O. BOX 174 SOUTH BERWICK, MAINE 03908 (207) 384-5587

CERTIFIED SOIL SCIENTIST

WETLAND SCIENTIST

LICENSED SITE EVALUATOR

January 22, 2025

Mr. Eric Weinrieb, P.E.
Altus Engineering
133 Court Street
Portsmouth, New Hampshire 03801

RE: 56 Ridges Court, Portsmouth, New Hampshire, JWN #23-142

Dear Eric:

Per your request, the following information is provided to assist you in the Conditional Use Permit Application requirements. Specifically, Section 10.101722(3) of the City Of Portsmouth, New Hampshire Zoning Ordinance.

The wetland delineation was conducted on December 21, 2023 (both tidal and freshwater wetlands). The delineation was conducted in accordance with the U.S. Army Corps of Engineers document Corps of Engineers Wetlands Delineation Manual, (1987) along with the required Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Northcentral and Northeast Region, (Version 2, January 2012). The wetland boundary was located by North Easterly Surveying. Mr. Marc Jacobs, Wetland Scientist #010, reviewed and confirmed the wetland delineation on February 20, 2024. The attached FEMA 100 year flood and extended flood hazard map from the town GIS database for the properties more closely represents the existing wetland system compared to other available resource maps.

The proposed project will not encroach into the 100 foot buffer of the tidal system (refer to photo of adjacent off-site tidal system). The freshwater wetland where buffer encroachment will occur is approximately an acre in size and would classify as a wet meadow with poorly drained soils. The portion of the delineated wetland on the subject properties is essentially a mowed lawn with some scattered sedges within the yard(s) and one large willow (Salix sp.). A few scattered willows were noted in the wetland off-site as well. An on-site was conducted on January 16, 2025 to collect data on the plants within the more natural portion of the wetland that was within the paper road. This area had been recently cut and there was not enough vegetation left to classify most of the herbaceous layer (refer to photo – the more snow covered areas are maintained paths within the wetland). Adjacent to the property line of 56 Ridges Court the few shrubs that were observed included: common buttonbush (Cephalanthus occidentalis), rambler rose (Rosa multiflora), glossy false buckthorn (Frangula alnus), European buckthorn (Rhamnus cathartica), and honeysuckle (Lonicera sp.). On the property, the only invasive plant was some

bittersweet (*Celastrus sp.*) that was growing in the garden with the planted blackberries (*Rubus sp.*). Per Altus Engineering "Site Preparation Plan" they plan to remove miscellaneous garden area features where the bittersweet is growing. The bittersweet should be carefully removed and properly disposed of. A request from the Natural Heritage Bureau (NHB) was conducted and no rare species or exemplary natural communities were documented on the property (refer to attachment). There was a NHB record nearby but the NHB determined the proposed project will not impact the NHB record (detailed information on the NHB record was not supplied). During the wetland flagging of the tidal wetland, Jesuits-bark (*Iva frutescens*) that is a state listed "Threatened" species was observed by the undersigned. These shrubs are off-site and will not be impacted by the proposed development.

A formal functions and values assessment is not required per Section 10.1017.22 of the City Of Portsmouth, New Hampshire Zoning Ordinance. Using professional judgement, the performance of the functions and values would be low due to: relatively small wetland size (1+/- acre), wetland is disturbed/routinely cut so vegetation is not diverse, subtle ditching within the wetland lowers the ability to store and slowly release water, and existing buffers around the wetland are developed with residential homes. This wet meadow is still of importance due to the nearby downstream tidal wetland system. Refer to Altus Engineering stormwater plan for details on protecting the wet meadow system from increased runoff, etc.

The proposed redevelopment of the property will reduce the driveway size, relocated the garage and the attached garden shed further from the wet meadow. There will be a proposed addition, new deck, etc. Refer to Altus Engineering plans for details on the existing versus proposed plans for the property. The impervious surface will increase with the proposed redevelopment of the property (refer to Altus Engineering plans for existing and proposed impervious surface area, and proposed effective impervious area figures). Per Altus Engineering, the compensation proposed is to consolidate Lot 68 & 69 with Lot 63. Plantings are discussed by Altus Engineering to offset the removal of trees and shrubs in the uplands. The actual plantings and locations will be determined by a landscape designer.

I hope this information is sufficient in the review of the proposed project. Please feel free to call with any questions.

Sincerely,

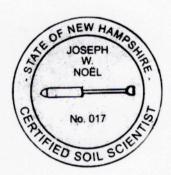
Joseph W. Noel

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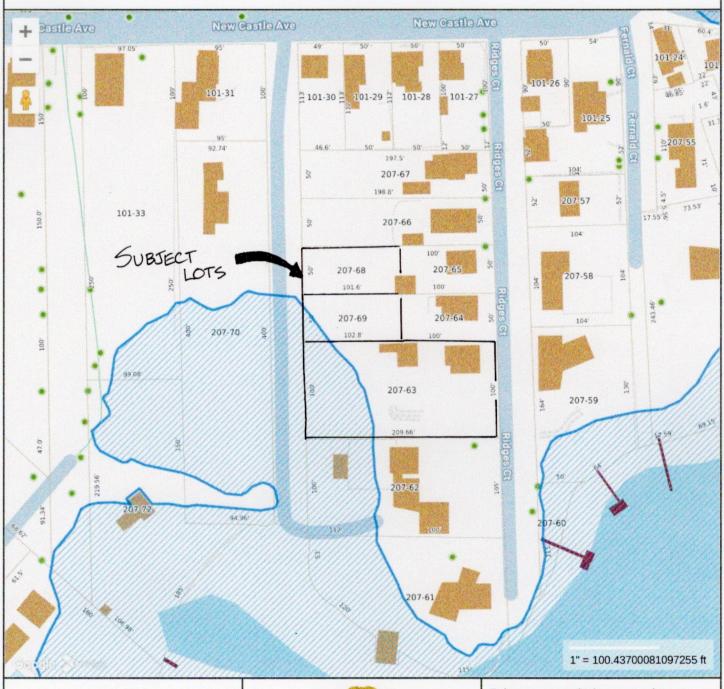
NH Certified Wetland Scientist #086

NH Certified Soil Scientist #017





56 Ridges Court - FEMA 100 year flood & Extended flood hazard area GIS layer





MAP FOR REFERENCE ONLY NOT A LEGAL DOCUMENT

City of Portsmouth, NH makes no claims and no warranties, expressed or implied, concerning the validity or accuracy of the GIS data presented on this map.

Geometry updated 09/26/2024

Print map scale is approximate. Critical layout or measurement activities should not be done using this resource.

PHOTOS

56 Ridges Court – Portsmouth, New Hampshire
(Photos taken by Joseph W. Noel on January 17, 2025)



Freshwater wetland system that was recently cut with berm in background and snow-covered maintained paths.



A view of the tidal wetland system with Canada geese taken from berm.

New Hampshire Natural Heritage Bureau NHB DataCheck Results Letter

To: Eric Weinrieb, Altus Engineering, Inc.

133 Court Street

Portsmouth, NH 03801

From: NH Natural Heritage Bureau

Date: 1/22/2025 (valid until 1/22/2026)

Re: Review by NH Natural Heritage Bureau of request submitted 1/6/2025

Permits: MUNICIPAL POR - Local Review, NHDES - Shoreland Standard Permit

NHB ID: NHB25-0048 Applicant: Trustees of Rainboth

Revocable Trust of 2010

Location: Portsmouth

56 Ridges Court

Project

Description: Proposed addition to the house, deck, and shed.

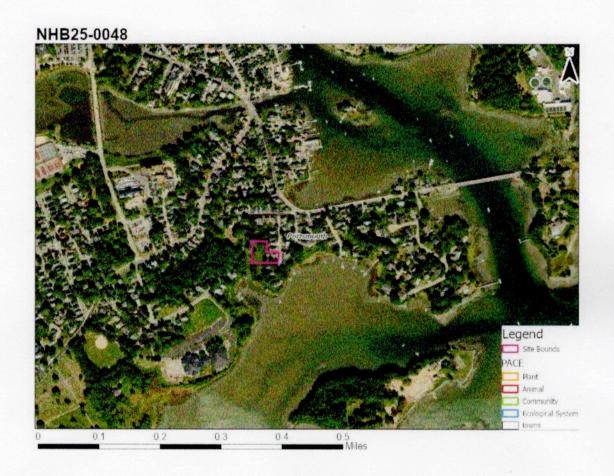
The NH Natural Heritage database has been checked by staff of the NH Natural Heritage Bureau and/or the NH Nongame and Endangered Species Program for records of rare species and exemplary natural communities near the area mapped below. The species considered include those listed as Threatened or Endangered by either the state of New Hampshire or the federal government.

It was determined that, although there was a NHB record (e.g., rare wildlife, plant, and/or natural community) present in the vicinity, we do not expect that it will be impacted by the proposed project. This determination was made based on the project information submitted via the NHB Datacheck Tool on 1/6/2025 5:30:44 P.M., and cannot be used for any other project.

Based on the information submitted, no further consultation with the NH Fish and Game Department pursuant to Fis 1004 is required.

New Hampshire Natural Heritage Bureau NHB DataCheck Results Letter

MAP OF PROJECT BOUNDARIES FOR: NHB25-0048



DRAINAGE ANALYSIS

FOR

Trustees of Rainboth Revocable Trust of 2010

56 Ridges Court Portsmouth, NH

Tax Map 207 Lots 63, 68, and 69

January 29, 2025

Stormwater Maintenance Manual revised 02/20/25

Prepared For:

Annmarie and Micheal Rainboth
Trustees of Rainboth Revocable Trust of 2010122 New Castle Avenue
Portsmouth, NH 03801

Prepared By:

ALTUS ENGINEERING

133 Court Street Portsmouth, NH 03801 Phone: (603) 433-2335





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Section 1

Narrative



PROJECT DESCRIPTION

The Trustees of the Rainboth Revocable Trust of 2010 are proposing to construct an addition to the existing home, a new driveway and a shed located at 56 Ridges Court Portsmouth, New Hampshire. The 0.71-acre property is identified as Tax Map 207, Lots 63, 68, and 69 and is located in the Single Residence-B District. The site is currently developed as a single-family residence. Access to the development site is via a driveway coming off Ridges Court.

The proposed project will construct a new addition, driveway, and shed. The house is serviced by municipal water and sewer. The proposed stormwater management system includes stone drip edges, a stone infiltration basin, and vegetative swales. These will mitigate and improve the storm water quality leaving the property.

Site Soils/Wetlands

Based off data from the USDA National Resources Conservation Service Web Soil Survey, the site sits on 799 Urban land-Canton complex soils. Altus recognizes these soils as HSG B and C except for the wetland which we categorized as HSG D based on poor infiltration capacity. Joseph W. Noel, Wetland Scientist, completed an on-site inspection on December 21, 2023, and identified a freshwater wetland greater than 10,000 square feet. This finding was confirmed by Wetlands Scientist, Marc Jacobs.

Pre-Development (Existing Conditions)

The site currently features a single-family home with a deck, detached shed, and paved driveway. Stormwater is collected in gutters around the home and is conveyed towards the wetland. The site generally slopes in a westerly direction towards the delineated wetland. Hydrology is characterized by two existing sub-catchments as delineated on the accompanying "Pre-Development Watershed Plan". Site runoff was analyzed at two points of analysis (POA). POA #1 is on the northern border of the property and POA #2 is in the southwest corner of the property under the wetland.

Post-Development (Proposed Conditions)

The site plan features the addition to the existing house as well as the new driveway and proposed shed.

The post-development conditions were analyzed at the same discharge point as the predevelopment conditions. The post-development watersheds are delineated on the accompanying "Post-Development Watershed Plan". Modifications to the delineated areas and associated ground cover were made to sub-catchments to account for the improvements to the property. As shown on the attached Post-Development Watershed Plan, the site was divided into seven postdevelopment sub-catchment areas. The same points of analysis in the Pre-Development model were used for comparison of the Pre- and Post-development conditions.

The Post-Development Watershed Plan illustrates the proposed stormwater management system. Site topography, existing features, proposed site improvements, proposed grading, drainage and erosion control measures are shown on the accompanying plans. Recommended erosion control measures are based upon the December 2008 edition of the "New Hampshire Stormwater Manual Volumes 1 through 3" prepared by NHDES and Comprehensive Environmental, Inc. as amended.

CALCULATION METHODS

The drainage study was completed using the USDA SCS TR-20 Method within the HydroCAD Stormwater Modeling System. Reservoir routing was performed with the Dynamic Storage Indication method with automated calculation of tailwater conditions. A Type III 24-hour rainfall distribution was utilized in analyzing the data for the 2, 10, 25 and 50 year - 24-hour storm events using rainfall data provided by the Northeast Regional Climate Center (NRCC). 15% was added to each storm event's rainfall data as required in the city or Portsmouth site plan review regulations. A time span of 0 to 24 hours was analyzed at 0.01-hour increments. Infiltration rates are based on the K_{sat} Values for New Hampshire soils.

Drainage Analysis

A complete summary of the drainage model is included in the appendix of this report. The following table compares pre- and post-development peak rates at the Points of Analysis identified on the plans for the 2, 10, 25 and 50-year storm events:

Stormwater Modeling Summary
Peak Q (cfs) for Type III 24-Hour Storm Events

	2-Yr Storm	10-Yr Storm	25-Yr Storm	50-Yr Storm
	(3.69 inch)	(5.59 inch)	(7.10 inch)	(8.50 inch)
POA #1				
Pre	0.04	0.10	0.16	0.22
Post	0.04	0.10	0.16	0.22
Change	0.00	0.00	0.00	0.00
POA #2				
Pre	1.39	2.75	3.88	4.94
Post	1.25	2.47	3.49	4.93
Change	-0.14	-0.28	-0.39	-0.01

As the above table demonstrates, the proposed peak rates of runoff at the point of analysis will be decreased or unchanged from the existing conditions for all analyzed storm events.

CONCLUSION

This proposed site redevelopment of property located at 56 Ridges Court Portsmouth, New Hampshire will have no adverse effect on abutting properties as a result of stormwater runoff or siltation. Post-construction peak rates of runoff from the site will be lower than or the same as the existing conditions for all analyzed storm events. The new stormwater management system will also provide appropriate treatment to runoff from the proposed on-site impervious surfaces. Appropriate steps will be taken to properly mitigate erosion and sedimentation using temporary and permanent Best Management Practices for sediment and erosion control.

Disclaimer

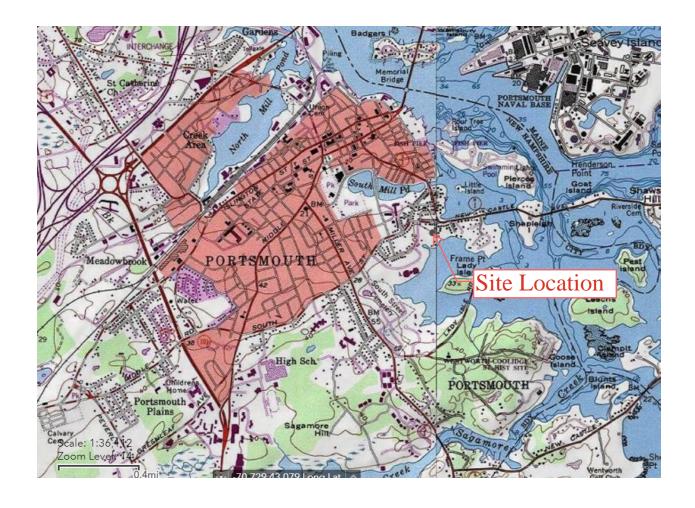
Altus Engineering, notes that stormwater modeling is limited in its capacity to precisely predict peak rates of runoff and flood elevations. Results should not be considered to represent actual storm events due to the number of variables and assumptions involved in the modeling effort. Surface roughness coefficients (n), entrance loss coefficients (ke), velocity factors (kv) and times of concentration (Tc) are based on subjective field observations and engineering judgment using available data. For design purposes, curve numbers (Cn) describe the average conditions. However, curve numbers will vary from storm to storm depending on the antecedent runoff conditions (ARC) including saturation and frozen ground. Also, higher water elevations than predicted by modeling could occur if drainage channels, closed drain systems or culverts are not maintained and/or become blocked by debris before and/or during a storm event as this will impact flow capacity of the structures. Structures should be re-evaluated if future changes occur within relevant drainage areas in order to assess any required design modifications.

Section 2

Aerial Photo and USGS Map







Section 3

Drainage Calculations

Pre-Development

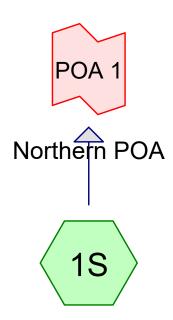
2-Year, 24-Hour Summary

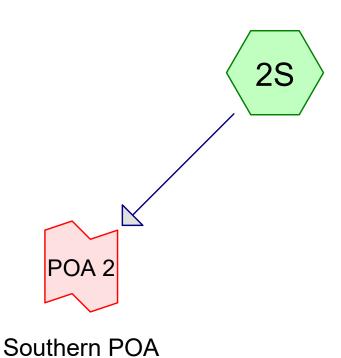
10-Year, 24-Hour Complete

25-Year, 24-Hour Summary

50-Year, 24-Hour Summary















Routing Diagram for 5639-HC-PRE-123024
Prepared by Altus Engineering, Printed 1/27/2025
HydroCAD® 10.00-26 s/n 01222 © 2020 HydroCAD Software Solutions LLC

Prepared by Altus Engineering
HydroCAD® 10.00-26 s/n 01222 © 2020 HydroCAD Software Solutions LLC

Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: Runoff Area=1,900 sf 0.00% Impervious Runoff Depth>0.85"

Tc=6.0 min CN=65 Runoff=0.04 cfs 0.003 af

Subcatchment2S: Runoff Area=34,047 sf 16.59% Impervious Runoff Depth>1.64"

Flow Length=248' Tc=8.0 min CN=78 Runoff=1.39 cfs 0.107 af

Link POA 1: Northern POA Inflow=0.04 cfs 0.003 af

Primary=0.04 cfs 0.003 af

Link POA 2: Southern POA Inflow=1.39 cfs 0.107 af

Primary=1.39 cfs 0.107 af

Type III 24-hr 25 Year Rainfall=7.10" Printed 1/27/2025

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: Runoff Area=1,900 sf 0.00% Impervious Runoff Depth>3.18"

Tc=6.0 min CN=65 Runoff=0.16 cfs 0.012 af

Subcatchment2S: Runoff Area=34,047 sf 16.59% Impervious Runoff Depth>4.56"

Flow Length=248' Tc=8.0 min CN=78 Runoff=3.88 cfs 0.297 af

Link POA 1: Northern POA Inflow=0.16 cfs 0.012 af

Primary=0.16 cfs 0.012 af

Link POA 2: Southern POA Inflow=3.88 cfs 0.297 af

Primary=3.88 cfs 0.297 af

Type III 24-hr 50 Year Rainfall=8.50" Printed 1/27/2025

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: Runoff Area=1,900 sf 0.00% Impervious Runoff Depth>4.30"

Tc=6.0 min CN=65 Runoff=0.22 cfs 0.016 af

Subcatchment2S: Runoff Area=34,047 sf 16.59% Impervious Runoff Depth>5.85"

Flow Length=248' Tc=8.0 min CN=78 Runoff=4.94 cfs 0.381 af

Link POA 1: Northern POA Inflow=0.22 cfs 0.016 af

Primary=0.22 cfs 0.016 af

Link POA 2: Southern POA Inflow=4.94 cfs 0.381 af

Primary=4.94 cfs 0.381 af

Printed 1/27/2025

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.033	61	>75% Grass cover, Good, HSG B (2S)
0.346	74	>75% Grass cover, Good, HSG C (2S)
0.181	80	>75% Grass cover, Good, HSG D (2S)
0.111	65	Brush, Good, HSG C (1S, 2S)
0.024	73	Brush, Good, HSG D (2S)
0.042	98	Paved parking, HSG B (2S)
0.043	98	Paved parking, HSG C (2S)
0.041	98	Roofs, HSG B (2S)
0.004	98	Roofs, HSG C (2S)
0.825	77	TOTAL AREA

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Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
0.115	HSG B	2S
0.505	HSG C	1S, 2S
0.205	HSG D	2S
0.000	Other	
0.825		TOTAL AREA

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Ground Covers (all nodes)

-	HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
	0.000	0.033	0.346	0.181	0.000	0.561	>75% Grass cover, Good	2S
	0.000	0.000	0.111	0.024	0.000	0.135	Brush, Good	1S, 2S
	0.000	0.042	0.043	0.000	0.000	0.085	Paved parking	2S
	0.000	0.041	0.004	0.000	0.000	0.045	Roofs	2S
	0.000	0.115	0.505	0.205	0.000	0.825	TOTAL AREA	

Printed 1/27/2025

Type III 24-hr 10 Year Rainfall=5.59" Printed 1/27/2025

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: Runoff Area=1,900 sf 0.00% Impervious Runoff Depth>2.05"

Tc=6.0 min CN=65 Runoff=0.10 cfs 0.007 af

Subcatchment2S: Runoff Area=34,047 sf 16.59% Impervious Runoff Depth>3.21"

Flow Length=248' Tc=8.0 min CN=78 Runoff=2.75 cfs 0.209 af

Link POA 1: Northern POA Inflow=0.10 cfs 0.007 af

Primary=0.10 cfs 0.007 af

Link POA 2: Southern POA Inflow=2.75 cfs 0.209 af

Primary=2.75 cfs 0.209 af

Total Runoff Area = 0.825 ac Runoff Volume = 0.217 af Average Runoff Depth = 3.15" 84.29% Pervious = 0.696 ac 15.71% Impervious = 0.130 ac

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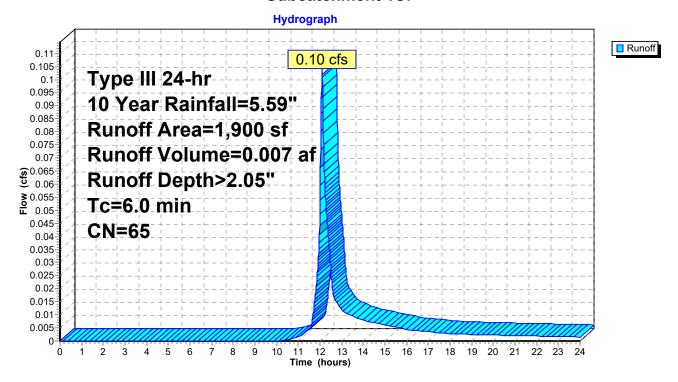
Summary for Subcatchment 1S:

Runoff = 0.10 cfs @ 12.09 hrs, Volume= 0.007 af, Depth> 2.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.59"

A	rea (sf)	CN [Description						
	1,900	65 E	Brush, Good, HSG C						
	1,900	1	100.00% Pervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Subcatchment 1S:



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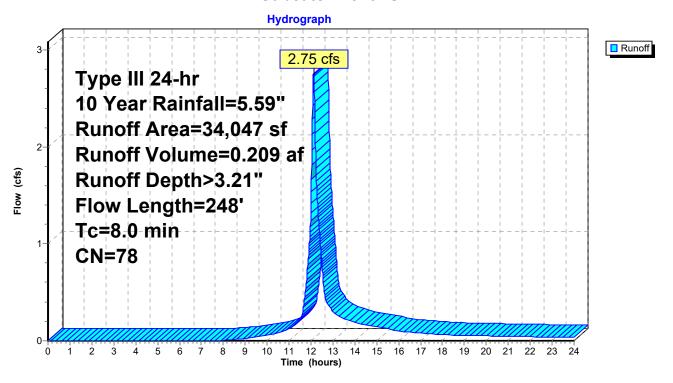
Summary for Subcatchment 2S:

Runoff = 2.75 cfs @ 12.11 hrs, Volume= 0.209 af, Depth> 3.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.59"

A	rea (sf)	CN [Description						
	1,767	98 F	Roofs, HSG B						
	195	98 F	Roofs, HSG C						
	1,811	98 F	Paved park	ing, HSG B					
	1,876	98 F	Paved parking, HSG C						
	1,445	61 >	75% Gras	s cover, Go	ood, HSG B				
	15,077			,	ood, HSG C				
	2,942		Brush, Goo						
	7,899				ood, HSG D				
	1,035	73 E	Brush, Goo	d, HSG D					
	34,047		Veighted A						
	28,398	3	33.41% Per	vious Area					
	5,649	1	6.59% Imp	pervious Ar	ea				
_									
Tc	Length	Slope		Capacity	Description				
(min)_	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.2	50	0.0800	0.16		Sheet Flow, Brush, HSG C				
0.0	400		0.04		n= 0.300 P2= 3.69"				
0.6	106	0.0377	2.91		Shallow Concentrated Flow,				
0.0	00	0.0400	0.70		Grassed Waterway Kv= 15.0 fps				
2.2	92	0.0100	0.70		Shallow Concentrated Flow, Brush, HSG D				
					Short Grass Pasture Kv= 7.0 fps				
8.0	248	Total							

Subcatchment 2S:



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Summary for Link POA 1: Northern POA

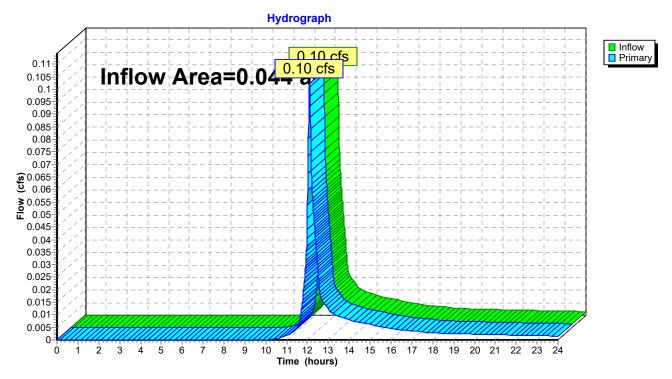
Inflow Area = 0.044 ac, 0.00% Impervious, Inflow Depth > 2.05" for 10 Year event

Inflow = 0.10 cfs @ 12.09 hrs, Volume= 0.007 af

Primary = 0.10 cfs @ 12.09 hrs, Volume= 0.007 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Link POA 1: Northern POA



Summary for Link POA 2: Southern POA

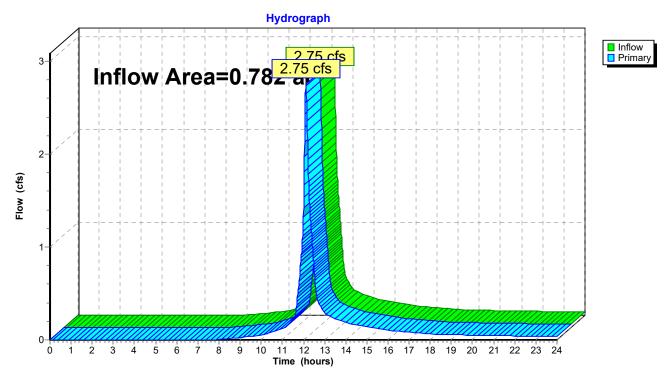
Inflow Area = 0.782 ac, 16.59% Impervious, Inflow Depth > 3.21" for 10 Year event

Inflow = 2.75 cfs @ 12.11 hrs, Volume= 0.209 af

Primary = 2.75 cfs @ 12.11 hrs, Volume= 0.209 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Link POA 2: Southern POA



Section 4

Drainage Calculations

Post-Development

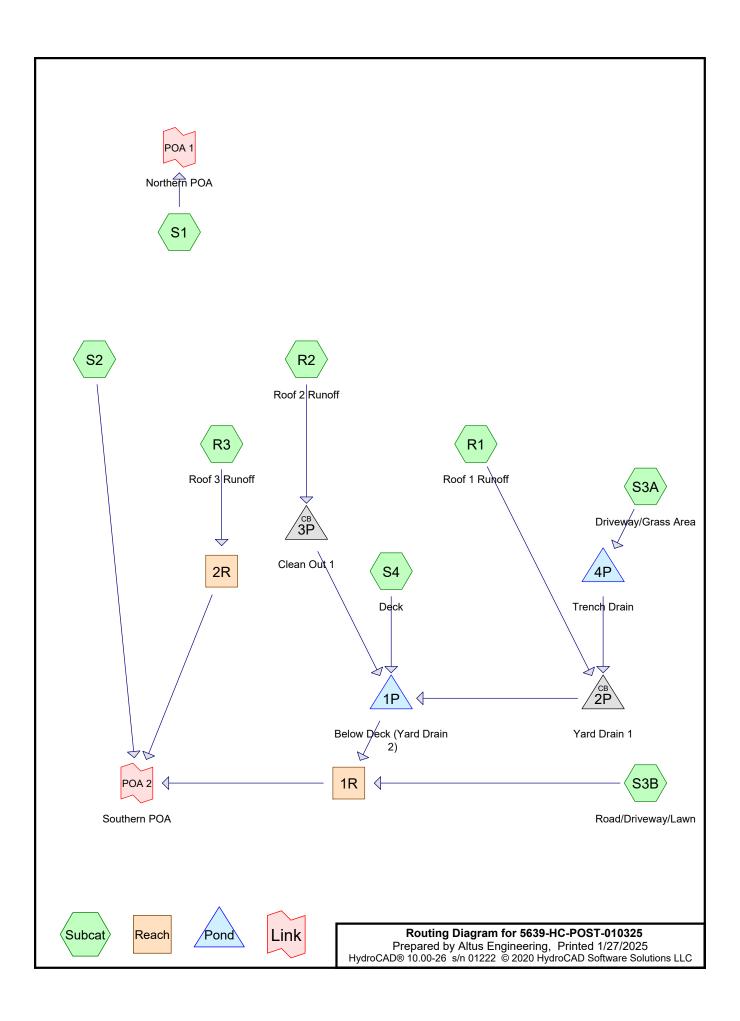
2-Year, 24-Hour Summary

10-Year, 24-Hour Complete

25-Year, 24-Hour Summary

50-Year, 24-Hour Summary





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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment R1: Roof 1 Runoff Runoff Runoff Area=1,149 sf 100.00% Impervious Runoff Depth>3.45"

Tc=6.0 min CN=98 Runoff=0.09 cfs 0.008 af

Subcatchment R2: Roof 2 Runoff Runoff Area=307 sf 100.00% Impervious Runoff Depth>3.45"

Tc=6.0 min CN=98 Runoff=0.03 cfs 0.002 af

Subcatchment R3: Roof 3 Runoff Runoff Area=476 sf 100.00% Impervious Runoff Depth>3.45"

Tc=6.0 min CN=98 Runoff=0.04 cfs 0.003 af

SubcatchmentS1: Runoff Area=1,900 sf 0.00% Impervious Runoff Depth>0.85"

Tc=6.0 min CN=65 Runoff=0.04 cfs 0.003 af

SubcatchmentS2: Runoff Area=26,434 sf 2.23% Impervious Runoff Depth>1.44"

Flow Length=248' Tc=8.0 min CN=75 Runoff=0.93 cfs 0.073 af

SubcatchmentS3A: Driveway/Grass Area Runoff Area=1,111 sf 65.35% Impervious Runoff Depth>2.18"

Tc=6.0 min CN=85 Runoff=0.07 cfs 0.005 af

SubcatchmentS3B: Road/Driveway/Lawn Runoff Area=3,576 sf 67.28% Impervious Runoff Depth>2.62"

Tc=6.0 min CN=90 Runoff=0.25 cfs 0.018 af

SubcatchmentS4: Deck Runoff Area=985 sf 15.74% Impervious Runoff Depth>2.72"

Tc=6.0 min CN=91 Runoff=0.07 cfs 0.005 af

Reach 1R: Avg. Flow Depth=0.06' Max Vel=1.28 fps Inflow=0.25 cfs 0.018 af

n=0.022 L=177.0' S=0.0169 '/' Capacity=10.11 cfs Outflow=0.24 cfs 0.018 af

Reach 2R: Avg. Flow Depth=0.02' Max Vel=0.11 fps Inflow=0.04 cfs 0.003 af

n=0.150 L=177.0' S=0.0282'/' Capacity=6.48 cfs Outflow=0.02 cfs 0.003 af

Pond 1P: Below Deck (Yard Drain 2) Peak Elev=10.23' Storage=176 cf Inflow=0.26 cfs 0.019 af

Discarded=0.06 cfs 0.019 af Primary=0.00 cfs 0.000 af Outflow=0.06 cfs 0.019 af

Pond 2P: Yard Drain 1 Peak Elev=10.23' Inflow=0.16 cfs 0.012 af

6.0" Round Culvert n=0.010 L=50.0' S=0.0020 '/' Outflow=0.16 cfs 0.012 af

Pond 3P: Clean Out 1 Peak Elev=12.09' Inflow=0.03 cfs 0.002 af

6.0" Round Culvert $\,$ n=0.010 L=70.0' S=0.0214 '/' Outflow=0.03 cfs 0.002 af

Pond 4P: Trench Drain Peak Elev=10.83' Storage=0.000 af Inflow=0.07 cfs 0.005 af

6.0" Round Culvert n=0.010 L=10.0' S=0.0580 '/' Outflow=0.07 cfs 0.005 af

Link POA 1: Northern POA Inflow=0.04 cfs 0.003 af

Primary=0.04 cfs 0.003 af

Link POA 2: Southern POA Inflow=1.19 cfs 0.094 af

Primary=1.19 cfs 0.094 af

5639-HC-POST-010325 Type III 24-hr 25 Year Rainfall=7.10" Printed 1/27/2025

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Runoff Area=1,149 sf 100.00% Impervious Runoff Depth>6.86" Subcatchment R1: Roof 1 Runoff

Tc=6.0 min CN=98 Runoff=0.18 cfs 0.015 af

Runoff Area=307 sf 100.00% Impervious Runoff Depth>6.86" Subcatchment R2: Roof 2 Runoff

Tc=6.0 min CN=98 Runoff=0.05 cfs 0.004 af

Runoff Area=476 sf 100.00% Impervious Runoff Depth>6.86" Subcatchment R3: Roof 3 Runoff

Tc=6.0 min CN=98 Runoff=0.08 cfs 0.006 af

Runoff Area=1,900 sf 0.00% Impervious Runoff Depth>3.18" Subcatchment S1:

Tc=6.0 min CN=65 Runoff=0.16 cfs 0.012 af

Runoff Area=26,434 sf 2.23% Impervious Runoff Depth>4.23" Subcatchment S2:

Flow Length=248' Tc=8.0 min CN=75 Runoff=2.81 cfs 0.214 af

Runoff Area=1,111 sf 65.35% Impervious Runoff Depth>5.34" Subcatchment S3A: Driveway/Grass Area

Tc=6.0 min CN=85 Runoff=0.16 cfs 0.011 af

Runoff Area=3,576 sf 67.28% Impervious Runoff Depth>5.92" Subcatchment S3B: Road/Driveway/Lawn

Tc=6.0 min CN=90 Runoff=0.54 cfs 0.040 af

Runoff Area=985 sf 15.74% Impervious Runoff Depth>6.03" Subcatchment S4: Deck

Tc=6.0 min CN=91 Runoff=0.15 cfs 0.011 af

Avg. Flow Depth=0.11' Max Vel=1.89 fps Inflow=0.74 cfs 0.046 af Reach 1R:

n=0.022 L=177.0' S=0.0169 '/' Capacity=10.11 cfs Outflow=0.69 cfs 0.046 af

Avg. Flow Depth=0.03' Max Vel=0.15 fps Inflow=0.08 cfs 0.006 af Reach 2R:

n=0.150 L=177.0' S=0.0282 '/' Capacity=6.48 cfs Outflow=0.05 cfs 0.006 af

Pond 1P: Below Deck (Yard Drain 2) Peak Elev=10.72' Storage=353 cf Inflow=0.54 cfs 0.042 af

Discarded=0.06 cfs 0.036 af Primary=0.33 cfs 0.006 af Outflow=0.40 cfs 0.042 af

Peak Elev=10.83' Inflow=0.34 cfs 0.026 af Pond 2P: Yard Drain 1

6.0" Round Culvert n=0.010 L=50.0' S=0.0020 '/' Outflow=0.34 cfs 0.026 af

Peak Elev=12.13' Inflow=0.05 cfs 0.004 af Pond 3P: Clean Out 1

6.0" Round Culvert n=0.010 L=70.0' S=0.0214 '/' Outflow=0.05 cfs 0.004 af

Peak Elev=10.94' Storage=0.000 af Inflow=0.16 cfs 0.011 af Pond 4P: Trench Drain

6.0" Round Culvert n=0.010 L=10.0' S=0.0580 '/' Outflow=0.16 cfs 0.011 af

Link POA 1: Northern POA Inflow=0.16 cfs 0.012 af

Primary=0.16 cfs 0.012 af

Link POA 2: Southern POA Inflow=3.37 cfs 0.266 af

Primary=3.37 cfs 0.266 af

Type III 24-hr 50 Year Rainfall=8.50" Printed 1/27/2025

5639-HC-POST-010325

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment R1: Roof 1 Runoff Runoff Runoff Area=1,149 sf 100.00% Impervious Runoff Depth>8.25"

Tc=6.0 min CN=98 Runoff=0.22 cfs 0.018 af

Subcatchment R2: Roof 2 Runoff Runoff Area=307 sf 100.00% Impervious Runoff Depth>8.25"

Tc=6.0 min CN=98 Runoff=0.06 cfs 0.005 af

Subcatchment R3: Roof 3 Runoff Runoff Area=476 sf 100.00% Impervious Runoff Depth>8.25"

Tc=6.0 min CN=98 Runoff=0.09 cfs 0.008 af

SubcatchmentS1: Runoff Area=1,900 sf 0.00% Impervious Runoff Depth>4.30"

Tc=6.0 min CN=65 Runoff=0.22 cfs 0.016 af

Subcatchment S2: Runoff Area=26,434 sf 2.23% Impervious Runoff Depth>5.49"

Flow Length=248' Tc=8.0 min CN=75 Runoff=3.63 cfs 0.277 af

SubcatchmentS3A: Driveway/Grass Area Runoff Area=1,111 sf 65.35% Impervious Runoff Depth>6.69"

Tc=6.0 min CN=85 Runoff=0.19 cfs 0.014 af

SubcatchmentS3B: Road/Driveway/Lawn Runoff Area=3,576 sf 67.28% Impervious Runoff Depth>7.29"

Tc=6.0 min CN=90 Runoff=0.65 cfs 0.050 af

SubcatchmentS4: Deck Runoff Area=985 sf 15.74% Impervious Runoff Depth>7.41"

Tc=6.0 min CN=91 Runoff=0.18 cfs 0.014 af

Reach 1R: Avg. Flow Depth=0.15' Max Vel=2.23 fps Inflow=1.18 cfs 0.060 af

n=0.022 L=177.0' S=0.0169 '/' Capacity=10.11 cfs Outflow=1.13 cfs 0.060 af

Reach 2R: Avg. Flow Depth=0.03' Max Vel=0.17 fps Inflow=0.09 cfs 0.008 af

n=0.150 L=177.0' S=0.0282 '/' Capacity=6.48 cfs Outflow=0.06 cfs 0.007 af

Pond 1P: Below Deck (Yard Drain 2) Peak Elev=10.73' Storage=361 cf Inflow=0.64 cfs 0.051 af

Discarded=0.06 cfs 0.041 af Primary=0.56 cfs 0.010 af Outflow=0.62 cfs 0.051 af

Pond 2P: Yard Drain 1 Peak Elev=10.99' Inflow=0.41 cfs 0.032 af

6.0" Round Culvert n=0.010 L=50.0' S=0.0020 '/' Outflow=0.41 cfs 0.032 af

Pond 3P: Clean Out 1 Peak Elev=12.14' Inflow=0.06 cfs 0.005 af

6.0" Round Culvert $\,$ n=0.010 L=70.0' S=0.0214 '/' Outflow=0.06 cfs $\,$ 0.005 af

Pond 4P: Trench Drain Peak Elev=11.06' Storage=0.000 af Inflow=0.19 cfs 0.014 af

6.0" Round Culvert n=0.010 L=10.0' S=0.0580 '/' Outflow=0.19 cfs 0.014 af

Link POA 1: Northern POA Inflow=0.22 cfs 0.016 af

Primary=0.22 cfs 0.016 af

Link POA 2: Southern POA Inflow=4.79 cfs 0.345 af

Primary=4.79 cfs 0.345 af

Printed 1/27/2025

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.023	61	>75% Grass cover, Good, HSG B (S2, S3A, S3B)
0.342	74	>75% Grass cover, Good, HSG C (S2, S3B)
0.181	80	>75% Grass cover, Good, HSG D (S2)
0.102	65	Brush, Good, HSG C (S1, S2)
0.024	73	Brush, Good, HSG D (S2)
0.019	90	Deck, HSG C (S4)
0.052	98	Paved parking, HSG B (R1, S3A, S3B)
0.021	98	Paved parking, HSG C (S2, S3B)
0.039	98	Roofs, HSG B (R1, R2, S3B)
0.022	98	Roofs, HSG C (R3, S2, S4)
0.825	78	TOTAL AREA

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Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
0.114	HSG B	R1, R2, S2, S3A, S3B
0.506	HSG C	R3, S1, S2, S3B, S4
0.205	HSG D	S2
0.000	Other	
0.825		TOTAL AREA

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Ground Covers (all nodes)

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
 (acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
0.000	0.023	0.342	0.181	0.000	0.547	>75% Grass cover, Good	S2, S3A,
							S3B
0.000	0.000	0.102	0.024	0.000	0.126	Brush, Good	S1, S2
0.000	0.000	0.019	0.000	0.000	0.019	Deck	S4
0.000	0.052	0.021	0.000	0.000	0.073	Paved parking	R1, S2,
							S3A, S3B
0.000	0.039	0.022	0.000	0.000	0.060	Roofs	R1, R2,
							R3, S2,
							S3B, S4
0.000	0.114	0.506	0.205	0.000	0.825	TOTAL AREA	

Printed 1/27/2025

Type III 24-hr 10 Year Rainfall=5.59" Printed 1/27/2025

5639-HC-POST-010325

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment R1: Roof 1 Runoff Runoff Runoff Area=1,149 sf 100.00% Impervious Runoff Depth>5.35"

Tc=6.0 min CN=98 Runoff=0.14 cfs 0.012 af

Subcatchment R2: Roof 2 Runoff Runoff Area=307 sf 100.00% Impervious Runoff Depth>5.35"

Tc=6.0 min CN=98 Runoff=0.04 cfs 0.003 af

Subcatchment R3: Roof 3 Runoff Runoff Area=476 sf 100.00% Impervious Runoff Depth>5.35"

Tc=6.0 min CN=98 Runoff=0.06 cfs 0.005 af

Subcatchment S1: Runoff Area=1,900 sf 0.00% Impervious Runoff Depth>2.05"

Tc=6.0 min CN=65 Runoff=0.10 cfs 0.007 af

SubcatchmentS2: Runoff Area=26,434 sf 2.23% Impervious Runoff Depth>2.93"

Flow Length=248' Tc=8.0 min CN=75 Runoff=1.95 cfs 0.148 af

Subcatchment S3A: Driveway/Grass Area Runoff Area=1,111 sf 65.35% Impervious Runoff Depth>3.91"

Tc=6.0 min CN=85 Runoff=0.12 cfs 0.008 af

SubcatchmentS3B: Road/Driveway/Lawn Runoff Area=3,576 sf 67.28% Impervious Runoff Depth>4.44"

Tc=6.0 min CN=90 Runoff=0.41 cfs 0.030 af

Subcatchment S4: Deck Runoff Area = 985 sf 15.74% Impervious Runoff Depth > 4.55"

Tc=6.0 min CN=91 Runoff=0.11 cfs 0.009 af

Reach 1R: Avg. Flow Depth=0.08' Max Vel=1.54 fps Inflow=0.41 cfs 0.032 af

n=0.022 L=177.0' S=0.0169'/' Capacity=10.11 cfs Outflow=0.40 cfs 0.032 af

Reach 2R: Avg. Flow Depth=0.02' Max Vel=0.14 fps Inflow=0.06 cfs 0.005 af

n=0.150 L=177.0' S=0.0282 '/' Capacity=6.48 cfs Outflow=0.03 cfs 0.005 af

Pond 1P: Below Deck (Yard Drain 2) Peak Elev=10.71' Storage=341 cf Inflow=0.41 cfs 0.032 af

Discarded=0.06 cfs 0.030 af Primary=0.09 cfs 0.001 af Outflow=0.15 cfs 0.032 af

Pond 2P: Yard Drain 1 Peak Elev=10.73' Inflow=0.26 cfs 0.020 af

6.0" Round Culvert n=0.010 L=50.0' S=0.0020 '/' Outflow=0.26 cfs 0.020 af

Pond 3P: Clean Out 1 Peak Elev=12.11' Inflow=0.04 cfs 0.003 af

6.0" Round Culvert $\,$ n=0.010 L=70.0' S=0.0214 '/' Outflow=0.04 cfs 0.003 af

Pond 4P: Trench Drain Peak Elev=10.88' Storage=0.000 af Inflow=0.12 cfs 0.008 af

6.0" Round Culvert n=0.010 L=10.0' S=0.0580 '/' Outflow=0.12 cfs 0.008 af

Link POA 1: Northern POA Inflow=0.10 cfs 0.007 af

Primary=0.10 cfs 0.007 af

Link POA 2: Southern POA Inflow=2.37 cfs 0.185 af

Primary=2.37 cfs 0.185 af

Type III 24-hr 10 Year Rainfall=5.59" Printed 1/27/2025

5639-HC-POST-010325

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Total Runoff Area = 0.825 ac Runoff Volume = 0.223 af Average Runoff Depth = 3.24" 83.84% Pervious = 0.692 ac 16.16% Impervious = 0.133 ac

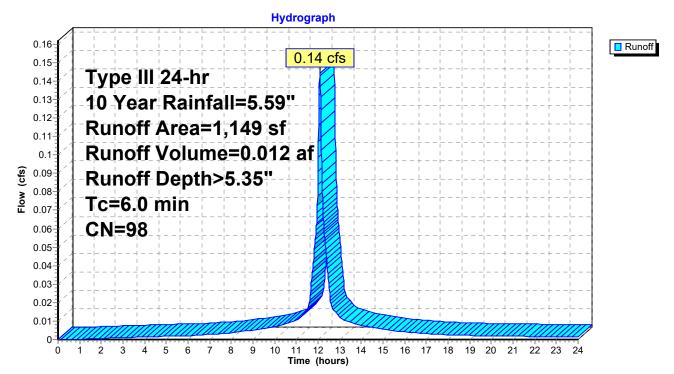
Summary for Subcatchment R1: Roof 1 Runoff

Runoff = 0.14 cfs @ 12.08 hrs, Volume= 0.012 af, Depth> 5.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.59"

_	Α	rea (sf)	CN	Description								
		1,071	98	Roofs, HSG B								
_		78	98	Paved park	Paved parking, HSG B							
		1,149	98	Weighted Average								
		1,149		100.00% Impervious Area								
	Tc	Length	Slope	e Velocity Capacity Description								
	(min)	(feet)	(ft/ft	(t) (ft/sec) (cfs)								
	6.0					Direct Entry,						

Subcatchment R1: Roof 1 Runoff



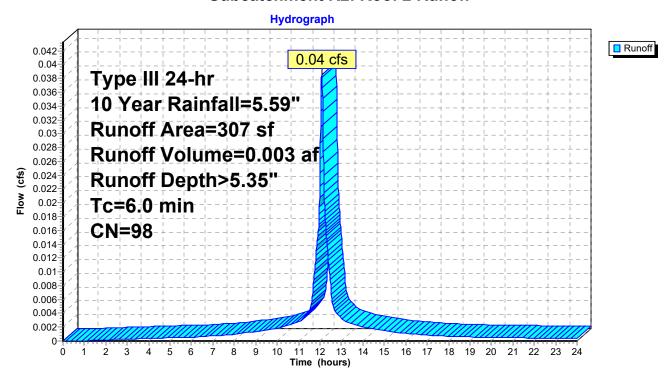
Summary for Subcatchment R2: Roof 2 Runoff

Runoff = 0.04 cfs @ 12.08 hrs, Volume= 0.003 af, Depth> 5.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.59"

 Α	rea (sf)	CN I	Description						
	307	98 I	Roofs, HSG B						
	307	•	100.00% In	npervious A	Area				
 Tc (min)	Length (feet)	•	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)						
6.0					Direct Entry,				

Subcatchment R2: Roof 2 Runoff



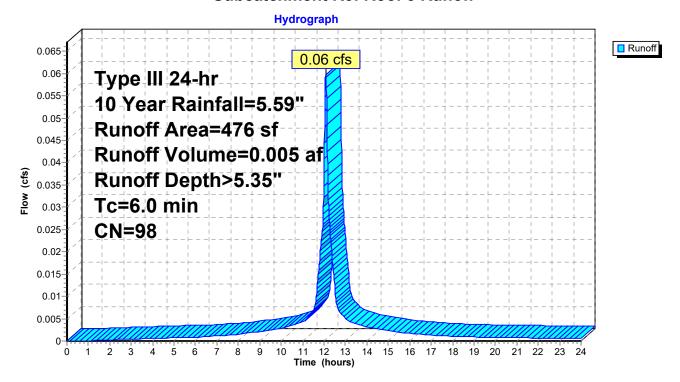
Summary for Subcatchment R3: Roof 3 Runoff

Runoff = 0.06 cfs @ 12.08 hrs, Volume= 0.005 af, Depth> 5.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.59"

_	Α	rea (sf)	CN I	Description						
		476	98 F	Roofs, HSG C						
		476		100.00% Impervious Area						
	Tc (min)	Length (feet)								
	6.0					Direct Entry,				

Subcatchment R3: Roof 3 Runoff



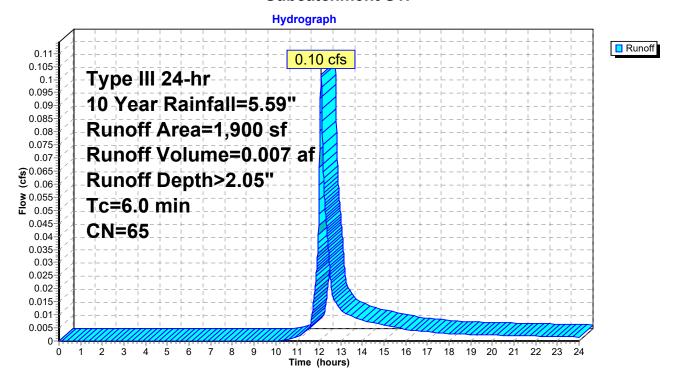
Summary for Subcatchment S1:

Runoff = 0.10 cfs @ 12.09 hrs, Volume= 0.007 af, Depth> 2.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.59"

A	rea (sf)	CN [Description						
	1,900	65 E	Brush, Good, HSG C						
	1,900	1	100.00% Pervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Subcatchment S1:



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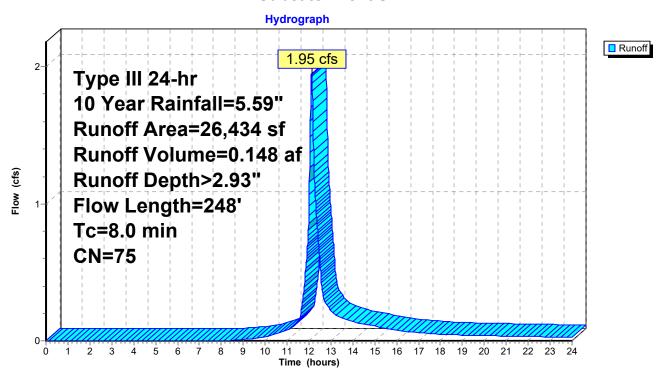
Summary for Subcatchment S2:

Runoff 1.95 cfs @ 12.12 hrs, Volume= 0.148 af, Depth> 2.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.59"

	Α	rea (sf)	CN	Description							
		320	98	Roofs, HSG C							
		270	98	Paved park	ing, HSG C						
		464	61	>75% Gras	s cover, Go	ood, HSG B					
		13,894	74	>75% Grass	s cover, Go	ood, HSG C					
		2,552	65	Brush, Goo	d, HSG C						
		7,899	80	>75% Gras	s cover, Go	ood, HSG D					
_		1,035	73	Brush, Goo	d, HSG D						
		26,434	75	Weighted A	verage						
		25,844		97.77% Per	vious Area						
		590		2.23% Impe	ervious Area	a					
	Tc	Length	Slope	•	Capacity	Description					
_	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)						
	5.2	50	0.0800	0.16		Sheet Flow, Brush, HSG C					
						n= 0.300 P2= 3.69"					
	0.6	106	0.037	7 2.91		Shallow Concentrated Flow,					
		•				Grassed Waterway Kv= 15.0 fps					
	2.2	92	0.0100	0.70		Shallow Concentrated Flow,					
_						Short Grass Pasture Kv= 7.0 fps					
	8.0	248	Total								

Subcatchment S2:



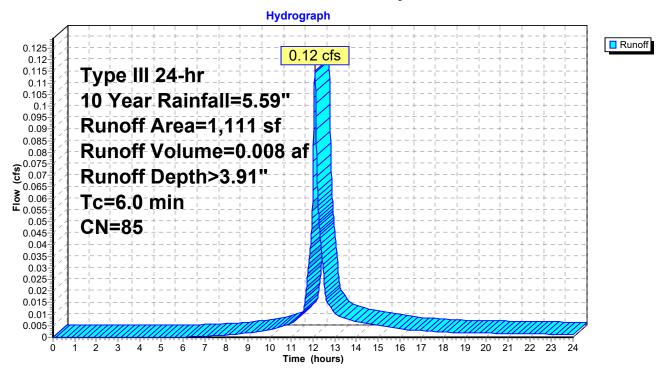
Summary for Subcatchment S3A: Driveway/Grass Area

Runoff = 0.12 cfs @ 12.09 hrs, Volume= 0.008 af, Depth> 3.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.59"

A	rea (sf)	CN	Description							
	726	98	Paved park	ing, HSG B	В					
	385	61	>75% Gras	s cover, Go	Good, HSG B					
	1,111	85	Weighted A	verage						
	385		34.65% Per	vious Area	a					
	726		65.35% Imp	ervious Are	rea					
Тс	Length	Slope	e Velocity Capacity Description							
(min)	(feet)	(ft/ft)	t) (ft/sec) (cfs)							
6.0			Direct Entry,							

Subcatchment S3A: Driveway/Grass Area



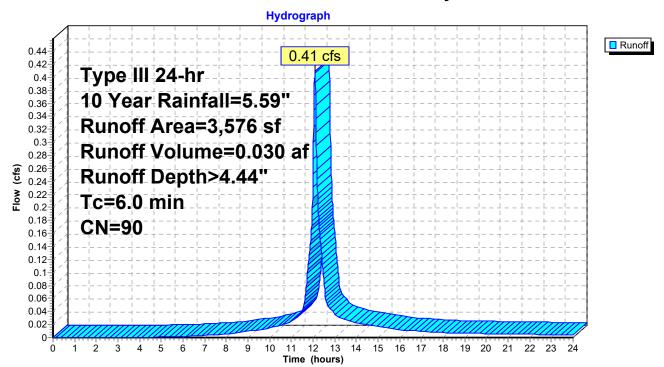
Summary for Subcatchment S3B: Road/Driveway/Lawn

Runoff = 0.41 cfs @ 12.08 hrs, Volume= 0.030 af, Depth> 4.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.59"

A	rea (sf)	CN	Description			
	301	98	Roofs, HSG	ВB		
	1,461	98	Paved park	ing, HSG B	}	
	644	98	Paved park	ing, HSG C	,	
	161	61	>75% Gras	s cover, Go	ood, HSG B	
	1,009	74	>75% Gras	s cover, Go	ood, HSG C	
	3,576	90	Weighted A	verage		
	1,170		32.72% Per	vious Area		
	2,406		67.28% Imp	ervious Ar	ea	
Tc	Length	Slop	e Velocity	Capacity	Description	
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)		
6.0					Direct Entry,	

Subcatchment S3B: Road/Driveway/Lawn



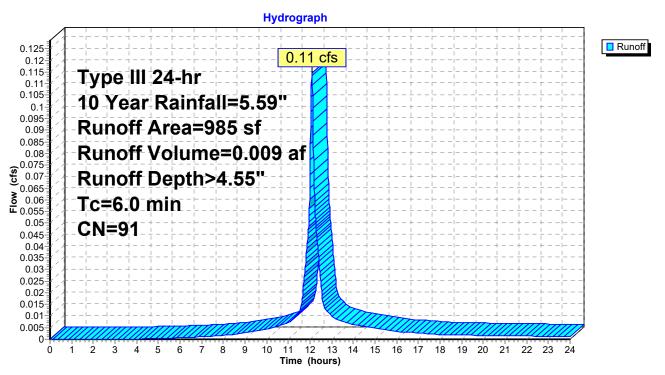
Summary for Subcatchment S4: Deck

Runoff = 0.11 cfs @ 12.08 hrs, Volume= 0.009 af, Depth> 4.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.59"

	Α	rea (sf)	CN	Description								
		155	98	Roofs, HSG	Roofs, HSG C							
*		830	90	Deck, HSG	Deck, HSG C							
		985	91	Weighted A	eighted Average							
		830		84.26% Pervious Area								
		155		15.74% Imp	ervious Are	rea						
	Тс	Length	Slope	e Velocity	Capacity	Description						
(n	nin)	(feet)	(ft/ft	t) (ft/sec) (cfs)								
	6.0					Direct Entry,						

Subcatchment S4: Deck



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Summary for Reach 1R:

Inflow Area = 0.164 ac, 66.54% Impervious, Inflow Depth > 2.33" for 10 Year event

Inflow = 0.41 cfs @ 12.08 hrs, Volume= 0.032 af

Outflow = 0.40 cfs @ 12.10 hrs, Volume= 0.032 af, Atten= 3%, Lag= 1.2 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Max. Velocity= 1.54 fps, Min. Travel Time= 1.9 min Avg. Velocity = 0.40 fps, Avg. Travel Time= 7.3 min

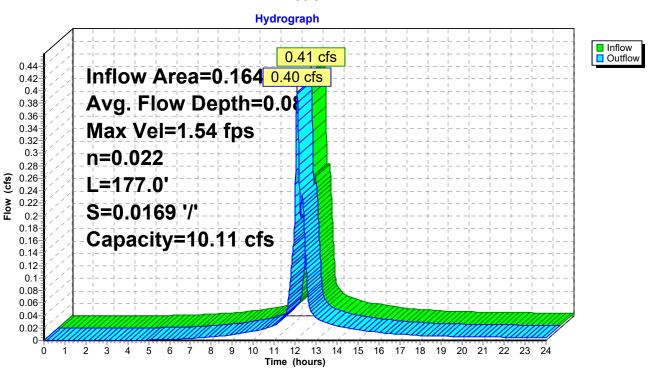
Peak Storage= 46 cf @ 12.10 hrs Average Depth at Peak Storage= 0.08'

Bank-Full Depth= 0.50' Flow Area= 2.3 sf, Capacity= 10.11 cfs

3.00' x 0.50' deep channel, n= 0.022 Grass Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 177.0' Slope= 0.0169 '/' Inlet Invert= 10.50', Outlet Invert= 7.50'



Reach 1R:



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Summary for Reach 2R:

Inflow Area = 0.011 ac,100.00% Impervious, Inflow Depth > 5.35" for 10 Year event

Inflow = 0.06 cfs @ 12.08 hrs, Volume= 0.005 af

Outflow = 0.03 cfs @ 12.20 hrs, Volume= 0.005 af, Atten= 43%, Lag= 6.7 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Max. Velocity= 0.14 fps, Min. Travel Time= 21.5 min Avg. Velocity = 0.05 fps, Avg. Travel Time= 54.8 min

Peak Storage= 44 cf @ 12.20 hrs Average Depth at Peak Storage= 0.02'

Bank-Full Depth= 0.50' Flow Area= 7.5 sf, Capacity= 6.48 cfs

10.00' x 0.50' deep channel, n= 0.150 Sheet flow over Short Grass

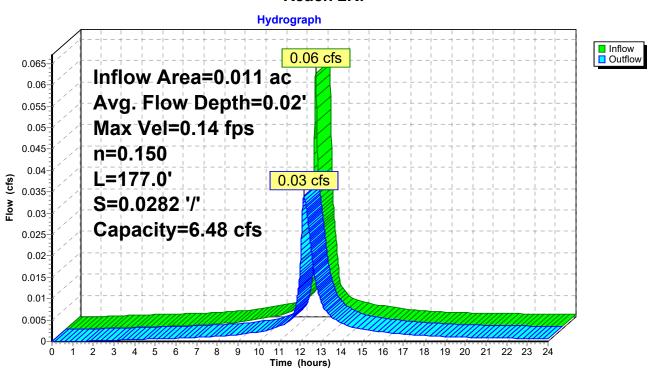
Side Slope Z-value= 10.0 '/' Top Width= 20.00'

Length= 177.0' Slope= 0.0282 '/'

Inlet Invert= 12.50', Outlet Invert= 7.50'



Reach 2R:



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Summary for Pond 1P: Below Deck (Yard Drain 2)

Inflow Area = 0.082 ac, 65.79% Impervious, Inflow Depth > 4.68" for 10 Year event Inflow 0.41 cfs @ 12.09 hrs, Volume= 0.032 af 0.15 cfs @ 12.33 hrs, Volume= Outflow 0.032 af, Atten= 63%, Lag= 14.7 min

0.06 cfs @ 12.33 hrs, Volume= Discarded = 0.030 af Primary 0.09 cfs @ 12.33 hrs, Volume= 0.001 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 10.71' @ 12.33 hrs Surf.Area= 830 sf Storage= 341 cf Flood Elev= 10.70' Surf.Area= 830 sf Storage= 332 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

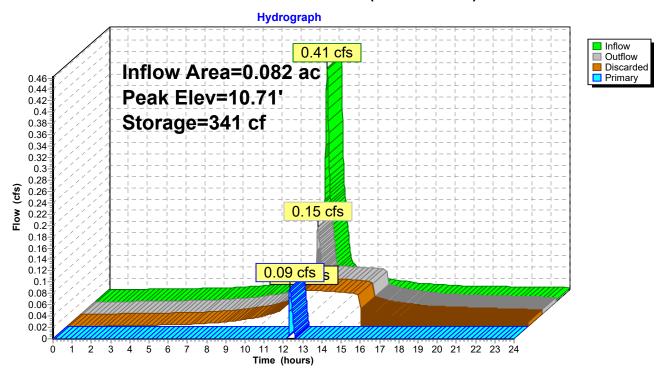
Center-of-Mass det. time= 29.4 min (800.2 - 770.8)

Volume	Inve	ert Ava	il.Storag	age Storage Description					
#1	9.7	0'	1,162 c	f Custom Stag	Custom Stage Data (Prismatic)Listed below (Recalc)				
Elevatio		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)				
9.7	70	830	0.0	0	0				
10.7	' 0	830	40.0	332	332				
11.7	70	830	100.0	830	1,162				
Device	Routing	In	vert O	utlet Devices					
#1	Discarde	d 9			tion over Surface				
2.50 3.00 3.50 4.00 4.50			readth Broad-Cre .40 0.60 0.80 1.0 00 4.50 5.00 5.50 4 2.50 2.70 2.68	sted Rectangular Weir 00 1.20 1.40 1.60 1.80 2.00 0 2.68 2.66 2.65 2.65 2.65					

Discarded OutFlow Max=0.06 cfs @ 12.33 hrs HW=10.71' (Free Discharge) 1=Exfiltration (Controls 0.06 cfs)

Primary OutFlow Max=0.09 cfs @ 12.33 hrs HW=10.71' TW=10.56' (Dynamic Tailwater) 2=Broad-Crested Rectangular Weir (Weir Controls 0.09 cfs @ 0.24 fps)

Pond 1P: Below Deck (Yard Drain 2)



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Summary for Pond 2P: Yard Drain 1

Inflow Area = 0.052 ac, 82.96% Impervious, Inflow Depth > 4.64" for 10 Year event

Inflow = 0.26 cfs @ 12.09 hrs, Volume= 0.020 af

Outflow = 0.26 cfs (a) 12.09 hrs, Volume= 0.020 af, Atten= 0%, Lag= 0.0 min

Primary = 0.26 cfs @ 12.09 hrs, Volume= 0.020 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

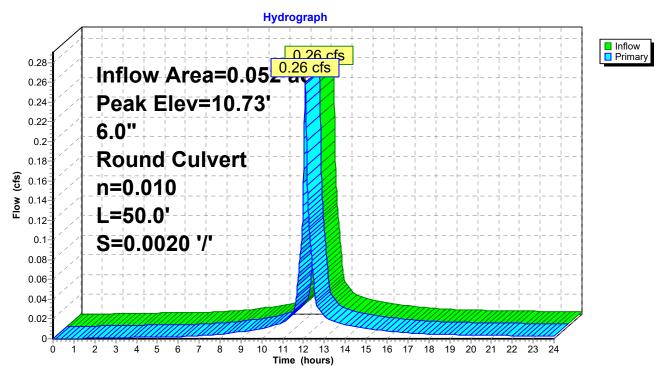
Peak Elev= 10.73' @ 12.32 hrs

Flood Elev= 11.10'

Device	Routing	Invert	Outlet Devices
#1	Primary	9.70'	6.0" Round Culvert L= 50.0' Ke= 0.500 Inlet / Outlet Invert= 9.70' / 9.60' S= 0.0020 '/' Cc= 0.900 n= 0.010 PVC, smooth interior. Flow Area= 0.20 sf

Primary OutFlow Max=0.20 cfs @ 12.09 hrs HW=10.32' TW=10.26' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.20 cfs @ 1.07 fps)

Pond 2P: Yard Drain 1



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Summary for Pond 3P: Clean Out 1

Inflow Area = 0.007 ac,100.00% Impervious, Inflow Depth > 5.35" for 10 Year event

Inflow = 0.04 cfs @ 12.08 hrs, Volume= 0.003 af

Outflow = 0.04 cfs @ 12.08 hrs, Volume= 0.003 af, Atten= 0%, Lag= 0.0 min

Primary = 0.04 cfs @ 12.08 hrs, Volume= 0.003 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

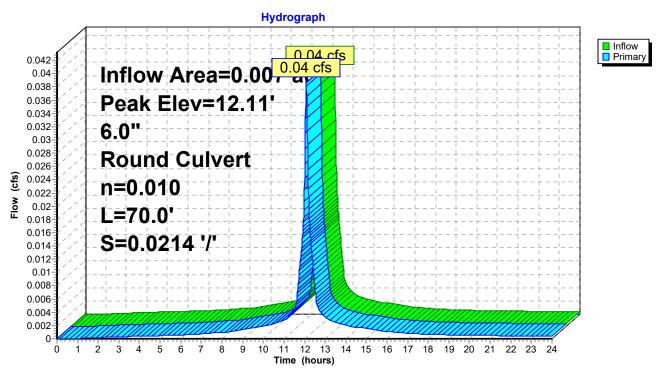
Peak Elev= 12.11' @ 12.08 hrs

Flood Elev= 15.25'

Device	Routing	Invert	Outlet Devices
#1	Primary	12.00'	6.0" Round Culvert L= 70.0' Ke= 0.500 Inlet / Outlet Invert= 12.00' / 10.50' S= 0.0214 '/' Cc= 0.900 n= 0.010 PVC smooth interior Flow Area= 0.20 sf

Primary OutFlow Max=0.04 cfs @ 12.08 hrs HW=12.11' TW=10.25' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.04 cfs @ 1.15 fps)

Pond 3P: Clean Out 1



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Summary for Pond 4P: Trench Drain

Inflow Area = 0.026 ac, 65.35% Impervious, Inflow Depth > 3.91" for 10 Year event

Inflow = 0.12 cfs @ 12.09 hrs, Volume= 0.008 af

Outflow = 0.12 cfs @ 12.09 hrs, Volume= 0.008 af, Atten= 0%, Lag= 0.2 min

Primary = 0.12 cfs @ 12.09 hrs, Volume= 0.008 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 10.88' @ 12.09 hrs Surf.Area= 0.000 ac Storage= 0.000 af

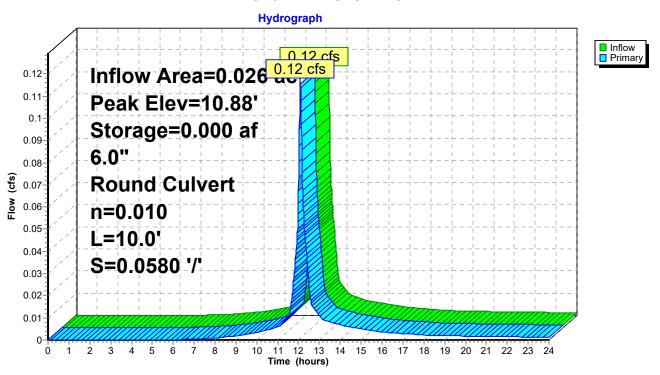
Flood Elev= 11.68' Surf.Area= 0.000 ac Storage= 0.000 af

Plug-Flow detention time= 1.2 min calculated for 0.008 af (100% of inflow) Center-of-Mass det. time= 0.8 min (803.4 - 802.6)

Volume	Invert	Avail.Storage	e Storage Description
#1	10.68'	0.000 a	af 0.50'W x 22.50'L x 1.00'H Prismatoid
Device	Routing	Invert (Outlet Devices
#1	Primary	ĺ	6.0" Round Culvert L= 10.0' Ke= 0.500 Inlet / Outlet Invert= 10.68' / 10.10' S= 0.0580 '/' Cc= 0.900 In= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.12 cfs @ 12.09 hrs HW=10.88' TW=10.33' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.12 cfs @ 1.54 fps)

Pond 4P: Trench Drain



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Summary for Link POA 1: Northern POA

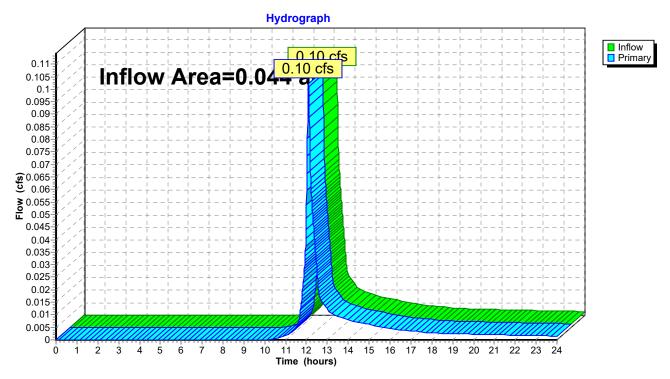
Inflow Area = 0.044 ac, 0.00% Impervious, Inflow Depth > 2.05" for 10 Year event

Inflow = 0.10 cfs @ 12.09 hrs, Volume= 0.007 af

Primary = 0.10 cfs @ 12.09 hrs, Volume= 0.007 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Link POA 1: Northern POA



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Summary for Link POA 2: Southern POA

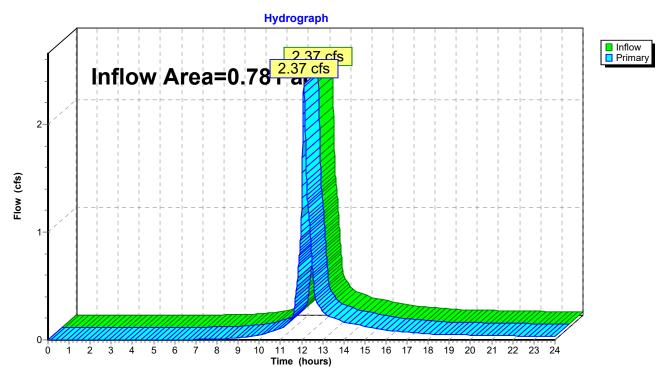
Inflow Area = 0.781 ac, 17.07% Impervious, Inflow Depth > 2.84" for 10 Year event

Inflow = 2.37 cfs @ 12.11 hrs, Volume= 0.185 af

Primary = 2.37 cfs @ 12.11 hrs, Volume= 0.185 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Link POA 2: Southern POA



Section 5

Precipitation Table



Extreme Precipitation Tables

Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Metadata for Point

Smoothing

State

Latitude Location

43.069 degrees North 70.75 degrees West

Longitude

Date/Time Elevation

Mon Dec 30 2024 12:29:14 GMT-0500 (Eastern Standard

values for modeling 15% added to

Extreme Precipitation Estimates

	•			1		2											
	5min		15min	10min 15min 30min 60min 120	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1 day	2d:
1yr	0.26	0.40	0.50	0.65	0.81	1.04	1yr	0.70	86.0	1.21	1.56	2.03	2.66	2.92	1yr	2.35	2.8
2yr	0.32	0.50	0.62	0.82	1.02	1.30	2yr	0.88	1.18	1.52	1.94	2.49	3.21	3.57	2yr	2.84	3.4
5yr	0.37	0.58	0.73	86.0	1.25	1.61	5yr	1.08	1.47	1.89	2.43	3.14	4.07	4.58	5yr	3.60	4.4
10yr	0.41	9.0	0.82	1.12	1.45	1.89	10yr	1.25	1.73	2.23	2.89	3.75	4.86	5.53	10yr	4.31	5.3
25yr	0.48	92.0	26.0	1.34	1.78	2.34	25yr	1.53	2.14	2.78	3.63	4.74	6.17	7.10	25yr	5.46	8.9
50yr	0.54	98.0	1.10	1.54	2.08	2.76	50yr	1.79	2.53	3.29	4.33	5.67	7.39	8.58	$50 \mathrm{yr}$	6.54	8.2
100yr	09.0	0.97	1.25	1.77	2.42	3.26	100yr	2.09	2.98	3.91	5.16	6.77	8.85	10.38	100yr	7.83	9.6
200yr	89.0	1.10	1.43	2.05	2.83	3.84	200yr	2.44	3.52	4.62	6.14	8.08	10.60	12.55	200yr	9:38	12.
500yr	08.0	1.32	1.72	2.49	3.49	4.78	500yr	3.01	4.39	5.78	7.72	10.22	13.47	16.14	500yr	11.92	15.

Lower Confidence Limits

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Section 6

NRCS Soils Report



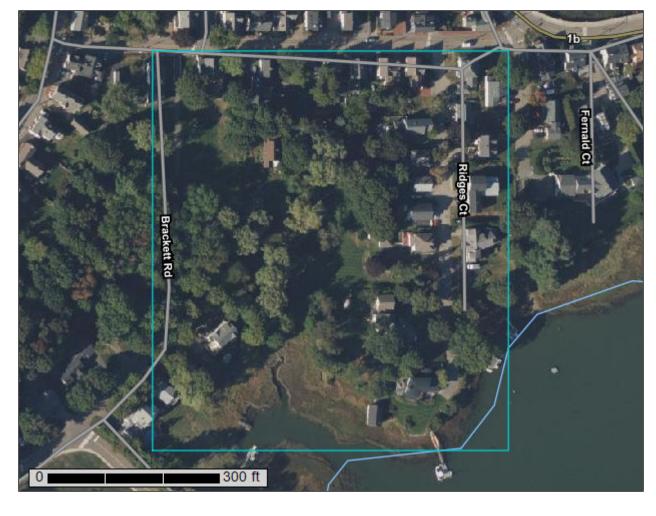


Natural Resources Conservation

Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for Rockingham County, New Hampshire



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2 053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



MAP LEGEND

Special Line Features Very Stony Spot Stony Spot Spoil Area Wet Spot Other Nater Features W 8 ◁ Soil Map Unit Polygons Area of Interest (AOI) Soil Map Unit Points Soil Map Unit Lines Special Point Features Area of Interest (AOI) Soils

Blowout 9

Streams and Canals

- **Borrow Pit** Clay Spot
- Closed Depression

Interstate Highways

Rails

ŧ

Fransportation

Major Roads Local Roads

US Routes

- **Gravel Pit**
- **Gravelly Spot**

Landfill

- Lava Flow
- Marsh or swamp

Aerial Photography

3ackground

Mine or Quarry

Miscellaneous Water

- Perennial Water
 - Rock Outcrop
- Saline Spot Sandy Spot
- Severely Eroded Spot
- Sinkhole

Slide or Slip

Sodic Spot

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

contrasting soils that could have been shown at a more detailed Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of

Please rely on the bar scale on each map sheet for map measurements. Source of Map: Natural Resources Conservation Service Coordinate System: Web Mercator (EPSG:3857) Web Soil Survey URL:

distance and area. A projection that preserves area, such as the Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Rockingham County, New Hampshire Version 27, Sep 3, 2024 Survey Area Data: Soil map units are labeled (as space allows) for map scales 1:50,000 or larger. Date(s) aerial images were photographed: Jun 19, 2020—Sep 20, 2020

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
799	Urban land-Canton complex, 3 to 15 percent slopes	9.3	93.0%
W	Water	0.7	7.0%
Totals for Area of Interest		10.0	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however,

onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An association is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Rockingham County, New Hampshire

799—Urban land-Canton complex, 3 to 15 percent slopes

Map Unit Setting

National map unit symbol: 9cq0 Elevation: 0 to 1,000 feet

Mean annual precipitation: 42 to 46 inches Mean annual air temperature: 45 to 48 degrees F

Frost-free period: 120 to 160 days

Farmland classification: Not prime farmland

Map Unit Composition

Urban land: 55 percent

Canton and similar soils: 20 percent Minor components: 25 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Canton

Setting

Parent material: Till

Typical profile

H1 - 0 to 5 inches: gravelly fine sandy loam H2 - 5 to 21 inches: gravelly fine sandy loam

H3 - 21 to 60 inches: loamy sand

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00

in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 5.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: A

Ecological site: F144AY034CT - Well Drained Till Uplands

Hydric soil rating: No

Minor Components

Udorthents

Percent of map unit: 5 percent Hydric soil rating: No

Boxford and eldridge

Percent of map unit: 4 percent

Hydric soil rating: No

Squamscott and scitico

Percent of map unit: 4 percent Landform: Marine terraces Hydric soil rating: Yes

Scituate and newfields

Percent of map unit: 4 percent Hydric soil rating: No

Chatfield

Percent of map unit: 4 percent Hydric soil rating: No

Walpole

Percent of map unit: 4 percent Landform: Depressions Hydric soil rating: Yes

W-Water

Map Unit Setting

National map unit symbol: 9cq3 Elevation: 200 to 2,610 feet

Farmland classification: Not prime farmland

Map Unit Composition

Water: 100 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

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Section 7

Stormwater Operations & Maintenance Plan



STORMWATER INSPECTION AND MAINTENANCE MANUAL

Trustees of Rainboth Revocable Trust of 2010

Tax Map 207, Lots 63, 68, and 69 56 Ridges Court Portsmouth, NH

OWNER:

Trustees of Rainboth Revocable Trust of 2010 122 New Castle Avenue Portsmouth, NH 03801

Proper inspection, maintenance, and repair are key elements in maintaining a successful stormwater management program on a developed property. Routine inspections ensure permit compliance and reduce the potential for deterioration of infrastructure or reduced water quality. The following responsible parties shall be in charge of managing the stormwater facilities:

RESPONSIBLE PARTIES:

Owner:	Micheal and An	603-431-1993	
	Name	Company	Phone
Inspection:		nnemarie Rainboth	603-431-1993
	Name	Company	Phone
Maintenance	: Micheal and Ar	nnemarie Rainboth	603-431-1993
	Name	Company	Phone

NOTES:

Written inspection forms and maintenance logs shall be completed yearly by a qualified inspector retained the owner or assigns.

Photographs of each stormwater BMP are to be taken at each inspection and submitted with the annual inspection reports.

Inspection and maintenance responsibilities shall transfer to any future property owner(s).

This manual shall be updated as needed to reflect any changes related to any transfer of ownership and/or any delegation of inspection and maintenance responsibilities to another entity



INFILTRATION BASINS

Function – Infiltration basins and tree box filters provide treatment to runoff prior to directing it to stormwater systems by filtering sediment and suspended solids, trapping them in the bottom of the facility and in the filter media itself. Additional treatment is provided by the native water-tolerant vegetation which removes nutrients and other pollutants through bio-uptake. Stormwater detention and infiltration can also be provided as the filtering process slows runoff, decreases the peak rate of discharge and promotes groundwater recharge.

Infiltration basin and tree box filters shall be managed (Per AGR 3800 and RSA 430:53) to: prevent and control the spread of invasive plant, insect, and fungal species; minimize the adverse environmental and economic effects invasive species cause to agriculture, forests, wetlands, wildlife, and other natural resources of the state; and protect the public from potential health problems attributed to certain invasive species.

Maintenance

- Inspect bi-annually and after significant rainfall events.
- If a infiltration basin or tree box filter does not completely drain within 72-hours following a rainfall event, then a qualified professional shall be retained to assess the condition of the facility to determine measures required to restore its filtration and/or infiltration function(s), including but not limited to removal of accumulated sediments and/or replacement or reconstruction of the filter media. Filter media shall be replaced with material matching the specification on the design drawings or the NHDES Stormwater Manual.
- Replace any riprap dislodged from spillways, inlets and outlets.
- Remove any obstructions, litter and accumulated sediment or debris as warranted but no less than once a year.
- Mowing of any grassed area in or adjacent to a raingarden or tree box filter, including any berms, shall be performed at least twice per year (when areas are not inundated) to keep the vegetation in vigorous condition. The cut grass shall be removed to prevent the decaying organic litter from clogging the filter media or choking other vegetation.
- Select vegetation should be maintained in healthy condition. This may include pruning, removal and replacement of dead or diseased vegetation.
- Remove any invasive species, Per AGR 3800 and RSA 430:53.
- Remove any hard wood growth aside from trees in tree box filters.
- Replace media in tree box filters when replacing tree.

CULVERTS AND DRAINAGE PIPES

Function – Culverts and drainage pipes convey stormwater away from buildings, walkways, and parking areas and to surface waters or closed drainage systems.

Maintenance

- Culverts and drainage pipes shall be inspected semi-annually, or more often as needed, for accumulation of debris and structural integrity. Leaves and other debris shall be removed from the inlet and outlet to insure the functionality of drainage structures. Debris shall be disposed of on site where it will not concentrate back at the drainage structures or at a solid waste disposal facility.
- Riprap Areas Culvert outlets and inlets shall be inspected during annual maintenance and operations for erosion and scour. If scour or creek erosion is identified, the outlet owner shall take appropriate means to prevent further erosion. Increased lengths of riprap may require a NHDES Permit and/or local permit.

CATCH BASINS/YARD DRAINS

Function – Catch basins and field drains collect stormwater, primarily from paved surfaces and roofs. Stormwater from paved areas often contains sediment and contaminants. Sumps serve to trap sediment, trace metals, nutrients and debris. Hooded catch basins trap hydrocarbons and floating debris.

Maintenance

- Remove leaves and debris from structure grates on an as-needed basis.
- Sumps shall be inspected and cleaned annually and any removed sediment and debris shall be disposed of at a solid waste disposal facility.

RIP RAP OUTLETS, SWALES AND PLUNGE POOLS

Function – Rip rap outlets slow the velocity of runoff, minimizing erosion and maximizing the treatment capabilities of associated buffers. Vegetated buffers, either forested or meadow, slow runoff which promotes and reduces peak rates of runoff. The reduced velocities and the presence of vegetation encourage the filtration of sediment and the limited bio-uptake of nutrients.

Maintenance

- Inspect riprap, level spreaders and buffers at least annually for signs of erosion, sediment buildup, or vegetation loss.
- Inspect level for signs of condensed flows. Level spreader and rip rap shall be maintained to disperse flows evenly over level spreader.
- If a meadow buffer, provide periodic mowing as needed to maintain a healthy stand of herbaceous vegetation.
- If a forested buffer, then the buffer should be maintained in an undisturbed condition, unless erosion occurs.
- If erosion of the buffer (forested or meadow) occurs, eroded areas should be repaired and replanted with vegetation similar to the remaining buffer. Corrective action should include eliminating the source of the erosion problem and may require retrofit or reconstruction of the level spreader.
- Remove debris and accumulated sediment and dispose of properly.

LANDSCAPED AREAS - FERTILIZER PROHIBITED

LANDSCAPED AREAS - LITTER CONTROL

Function – Landscaped areas tend to filter debris and contaminates that may block drainage systems and pollute the surface and ground waters.

Maintenance

- Litter Control and lawn maintenance involves removing litter such as trash, leaves, lawn clippings, pet wastes, oil and chemicals from streets, parking lots, and lawns before materials are transported into surface waters.
- Litter control shall be implemented as part of the grounds maintenance program.

VEGETATIVE SWALES

Function – Vegetative swales filter sediment from stormwater, promote infiltration, and the uptake of contaminates. They are designed to treat runoff and dispose of it safely into the natural drainage system.

Maintenance

- Timely maintenance is important to keep a swale in good working condition. Mowing of grassed swales shall be monthly to keep the vegetation in vigorous condition. The cut vegetation shall be removed to prevent the decaying organic litter from adding pollutants to the discharge from the swale.
- Fertilizing shall be bi-annual or as recommended from soil testing.
- Inspect swales following significant rainfall events.
- Woody vegetation shall not be allowed to become established in the swales or rock riprap outlet protection and if present shall be removed.
- Accumulated debris disrupts flow and leads to clogging and erosion. Remove debris and litter as necessary.
- Inspect for eroded areas. Determine cause of erosion and correct deficiency as required. Monitor repaired areas.

CONTROL OF INVASIVE PLANTS

Function – Invasive plants are introduced, alien, or non-native plants, which have been moved by people from their native habitat to a new area. Some exotic plants are imported for human use such as landscaping, erosion control, or food crops. They also can arrive as "hitchhikers" among shipments of other plants, seeds, packing materials, or fresh produce. Some exotic plants become invasive and cause harm by:

- becoming weedy and overgrown;
- killing established shade trees;
- obstructing pipes and drainage systems;
- forming dense beds in water;
- lowering water levels in lakes, streams, and wetlands;
- destroying natural communities;
- promoting erosion on stream banks and hillsides; and
- resisting control except by hazardous chemical.

Maintenance

During maintenance activities, check for the presence of invasive plants and remove in a safe manner as described in the attached "Methods for Disposing Non-Native Invasive Plants" prepared by the UNH Cooperative Extension.

GENERAL CLEAN UP

- Upon completion of the project, the contractor shall remove all temporary stormwater structures (i.e., temporary stone check dams, silt fence, temporary diversion swales, catch basin inlet filter, etc.). Any sediment deposits remaining in place after the silt fence or filter barrier is no longer required shall be dressed to conform to the existing grade, prepared, and seeded. Remove any sediment in catch basins and clean drain pipes that may have accumulated during construction.
- Once in operation, all paved areas of the site should be swept at least once annually at the end of winter/early spring prior to significant spring rains.

SNOW MANANGEMENT

Snow should never be stored in any stormwater practice as it may affect functionality by blocking drains and reducing the storage volume available for runoff. The Owner/Applicant and any maintenance personnel should take great care to ensure that snow is stored only in areas depicted on the site plan and away from locations that could negatively impact drainage infrastructure or flow paths.

PERMEABLE PAVERS

Function – Porous pavement (Pavers) is designed to capture rainwater runoff containing suspended solids, nutrients and pollutants. Proper maintenance of porous pavement is crucial for ensuring its longevity and functionality to infiltrate runoff.

Maintenance

- New porous pavement shall be inspected several times in the first month after construction and at least annually thereafter. Inspections shall be conducted after major storms to check for surface ponding that might indicate possible clogging.
- Inspect annually for pavement deterioration or spalling.
- Vacuum sweeping shall be performed once a year or as needed to maintain permeability.
 Power washing may be required prior to vacuum sweeping to dislodge trapped particles.
- Sand and abrasives shall not be used for winter maintenance, as they will clog the pores; de-icing materials shall be used instead.
- Never reseal or repave with impermeable materials. If the porous pavement is damaged, it can be repaired using conventional, non-porous patching mixes as long as the cumulative area repaired does not exceed 10 percent of the paved area.

APPPENDIX

- A. Stormwater System Operations and Maintenance Report
- B. Site Grading and Drainage Plan

STORM WATER SYSTEM OPERATION AND MAINTENANCE REPORT

General Information

Pro	ject Name			
Ow	ner			
Ins	pector's Name(s)			
Ins	pector's Contact Information			
Dat	te of Inspection		Start Time:	End Time:
	oe of Inspection: Annual Report Post-store	m event Due	to a discharge of significant amounts of sedim	ent
Not	tes:			
	C184- O		-L	
C L			charges of Significant Amounts of Sedin	ment
	oject	Status	Notes	
A discharge of significant amounts of sediment may be indicated by (but is not limited to) observations of the following. Note whether any are observed during this inspection: Notes/ Action taken:				
1	Do the current site conditions re	flect	Trotes/ Hetton taken	•
1	the attached site plan?	□No		
2	Is the site permanently stabilized			
	temporary erosion and sediment			
controls are removed, and stormwater		water		
	discharges from construction act	tivity		
	are eliminated?			
3	Is there evidence of the discharg			
	significant amounts of sediment			
	surface waters, or conveyance sy	ystems		
	leading to surface waters?			
		Permit	Coverage and Plans	
#	BMP/Facility	Inspected	Corrective Action Needed and Notes	Date Corrected
	Catch Basins	□Yes		
		□No		
	Drainage Pipes	□Yes		
	D: A/DI - D 1	□No		
	Riprap Aprons/Plunge Pools	□Yes □No		
	Site Vegetation	□Yes		
	Site vegetation	□No		
	Infiltration Basins	□Yes		
		□No		
	DICDECTED TO THE	DEDDE GEVER LETT	E DISTORDADING OF FLORE DATE.	NIGDE CEED AND

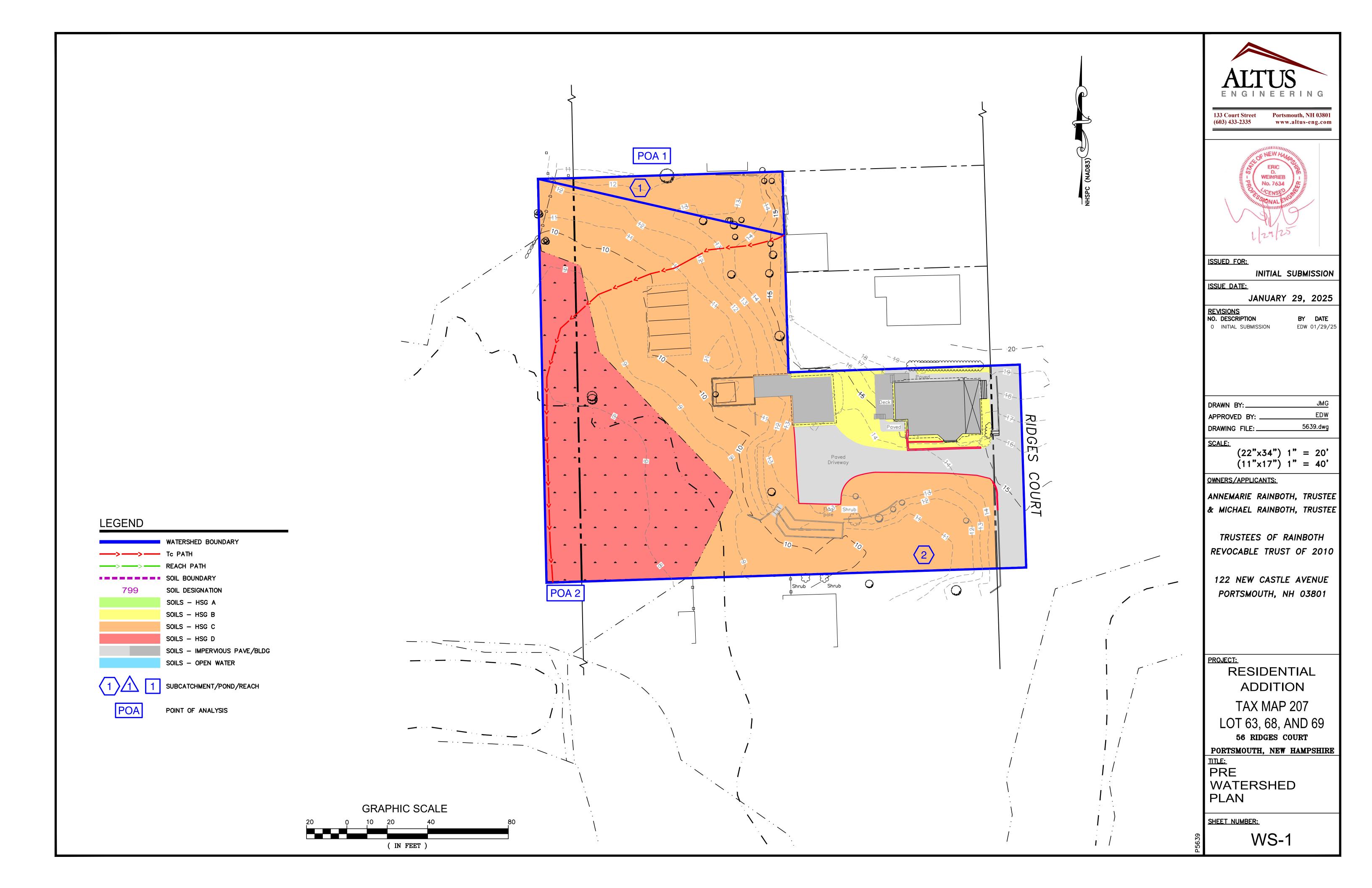
• INSPECTOR TO TAKE REPRESENTATIVE PHOTOGRAPHS OF EACH BMP INSPECTED AND INCLUDE THEM IN THE ANNUAL INPECTION REPORT.

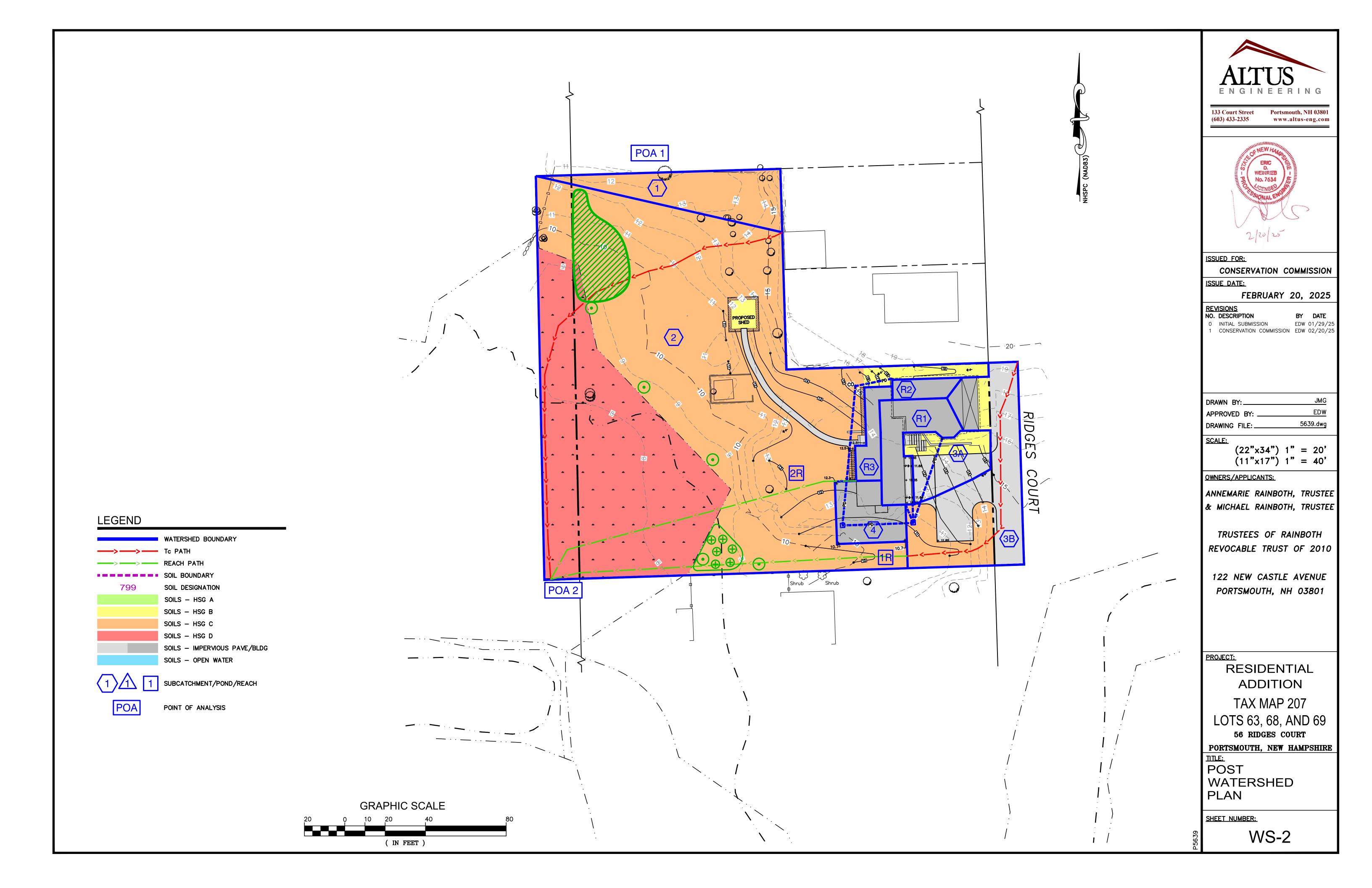
Section 8

Watershed Plans

Pre-Development Watershed Plan Post-Development Watershed Plan







PROPOSED ADDITION RAINBOTH RESIDENCE

56 Ridges Court Portsmouth, New Hampshire

Assessor's Parcel 207, Lots 63, 68, and 69

Owner/Applicant:

ANNEMARIE RAINBOTH, TRUSTEE & MICHAEL RAINBOTH, TRUSTEE

Plan Issue Date:

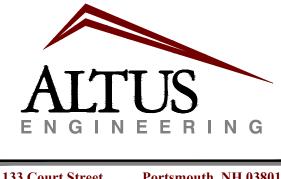
January 29, 2025 February 20, 2025

Conservation Commission (Initial Submission)
Conservation Commission

TRUSTEES OF RAINBOTH REVOCABLE TRUST OF 2010

122 New Castle Avenue Portsmouth, NH 03801 (603) 431-1993

Civil Engineer:



133 Court Street (603) 433-2335 Portsmouth, NH 03801 www.altus-eng.com

Surveyor:

North Easterly Surveying

SURVEYORS IN N.H. & MAINE 1021 Goodwin Road, Unit #1 Eliot, Maine 03903

(207) 439-6333

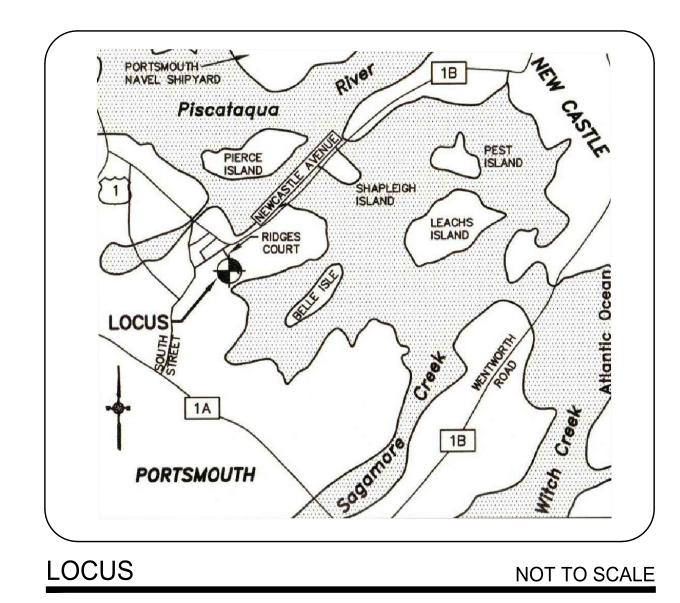
Building Designer: AMY DUTTON

9 Walker Street Kittery, ME 03904 (207) 345-6050

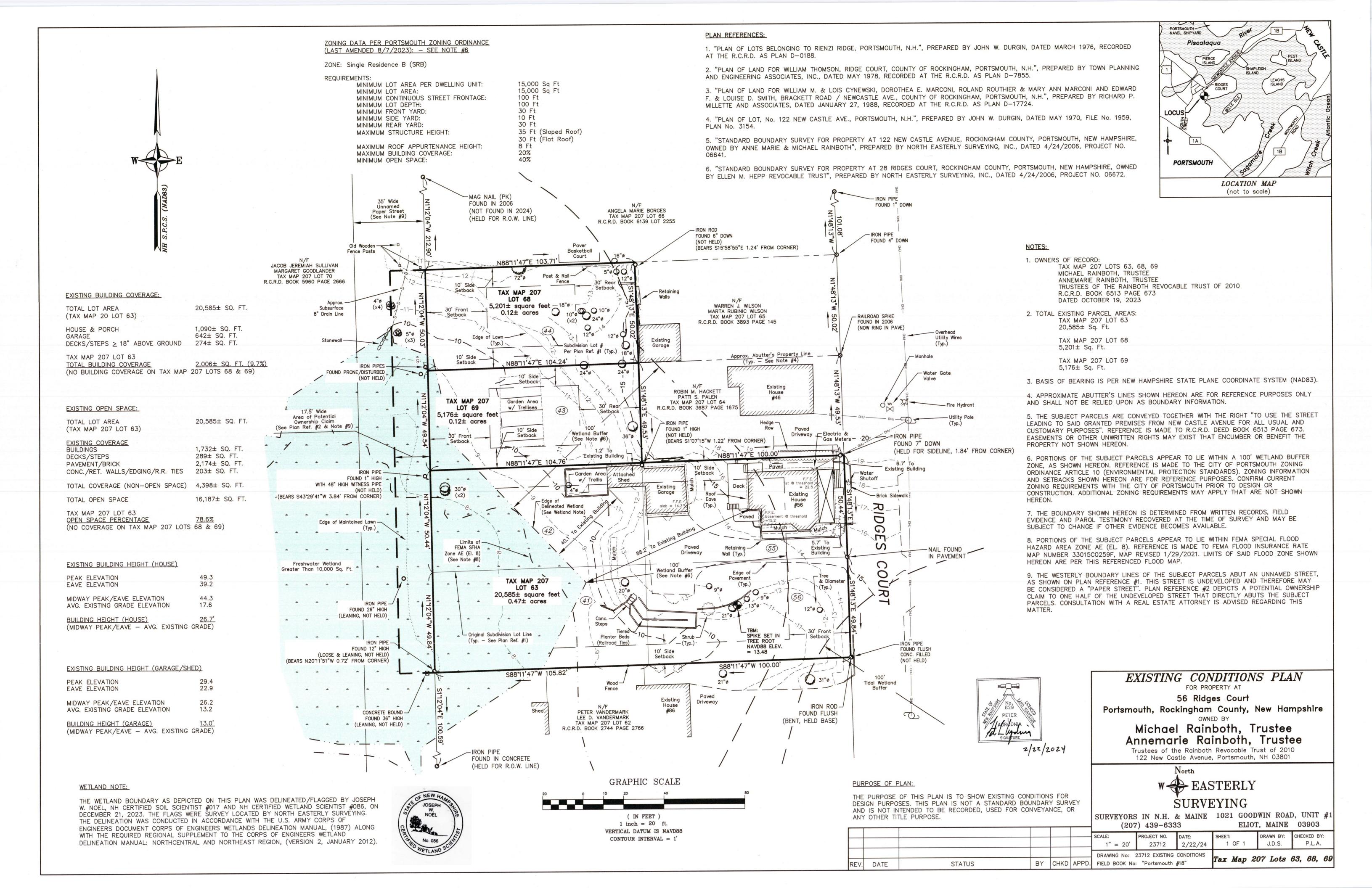
Wetland Scientist:

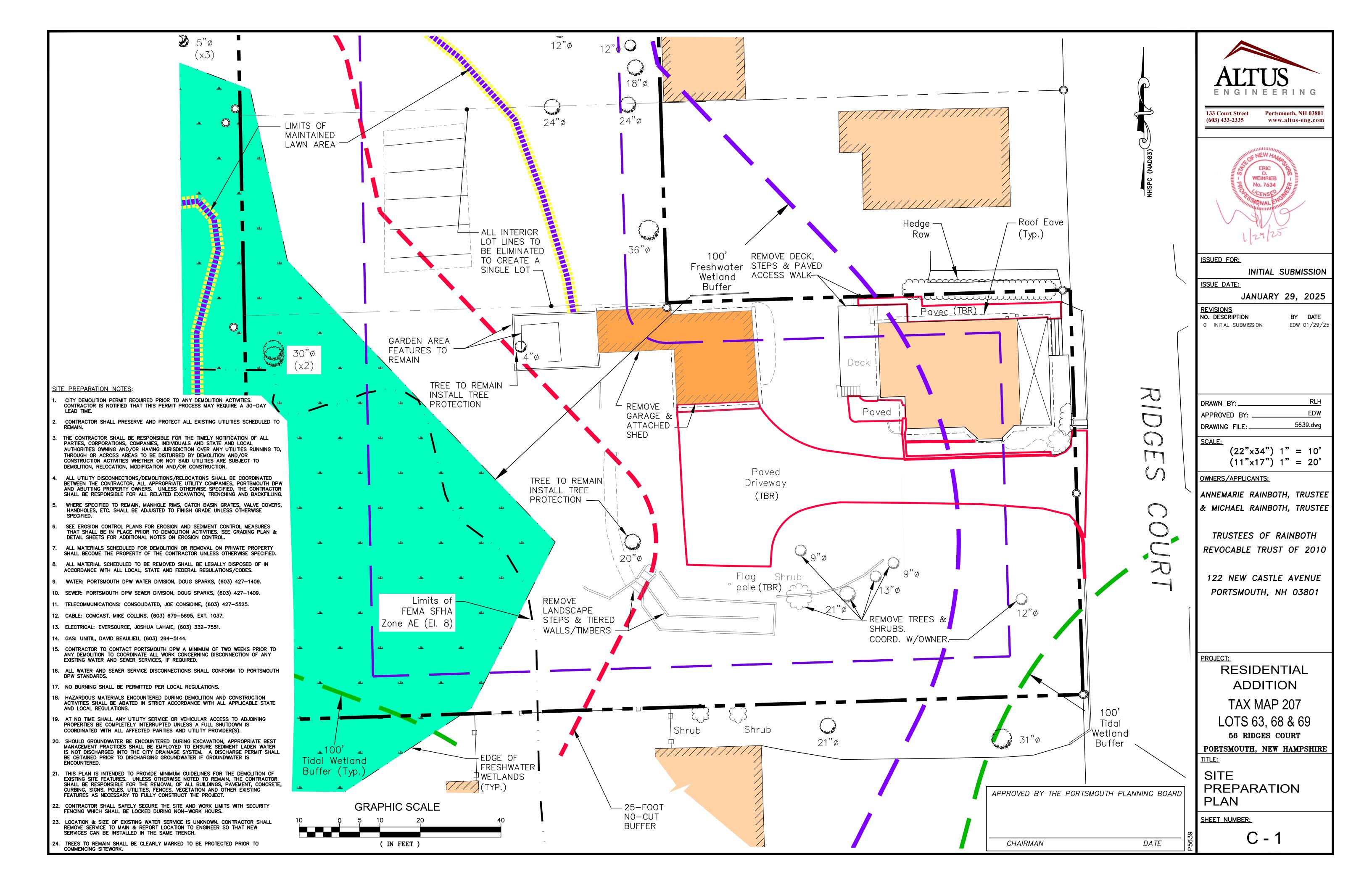
JOSEPH W. NOEL, NH CWS #086

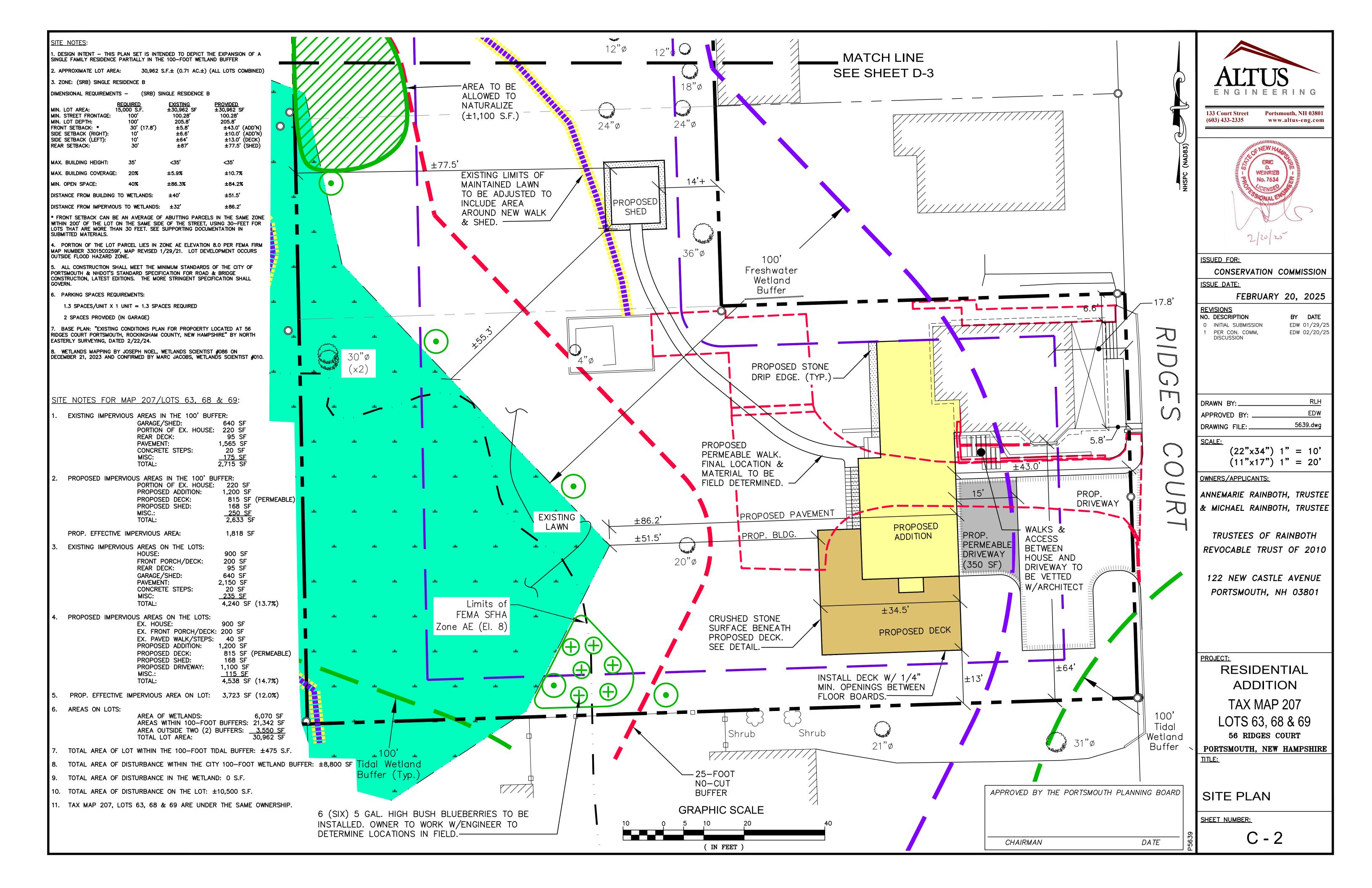
P.O. Box 174
South Berwick, ME 03908
(207) 384-5587

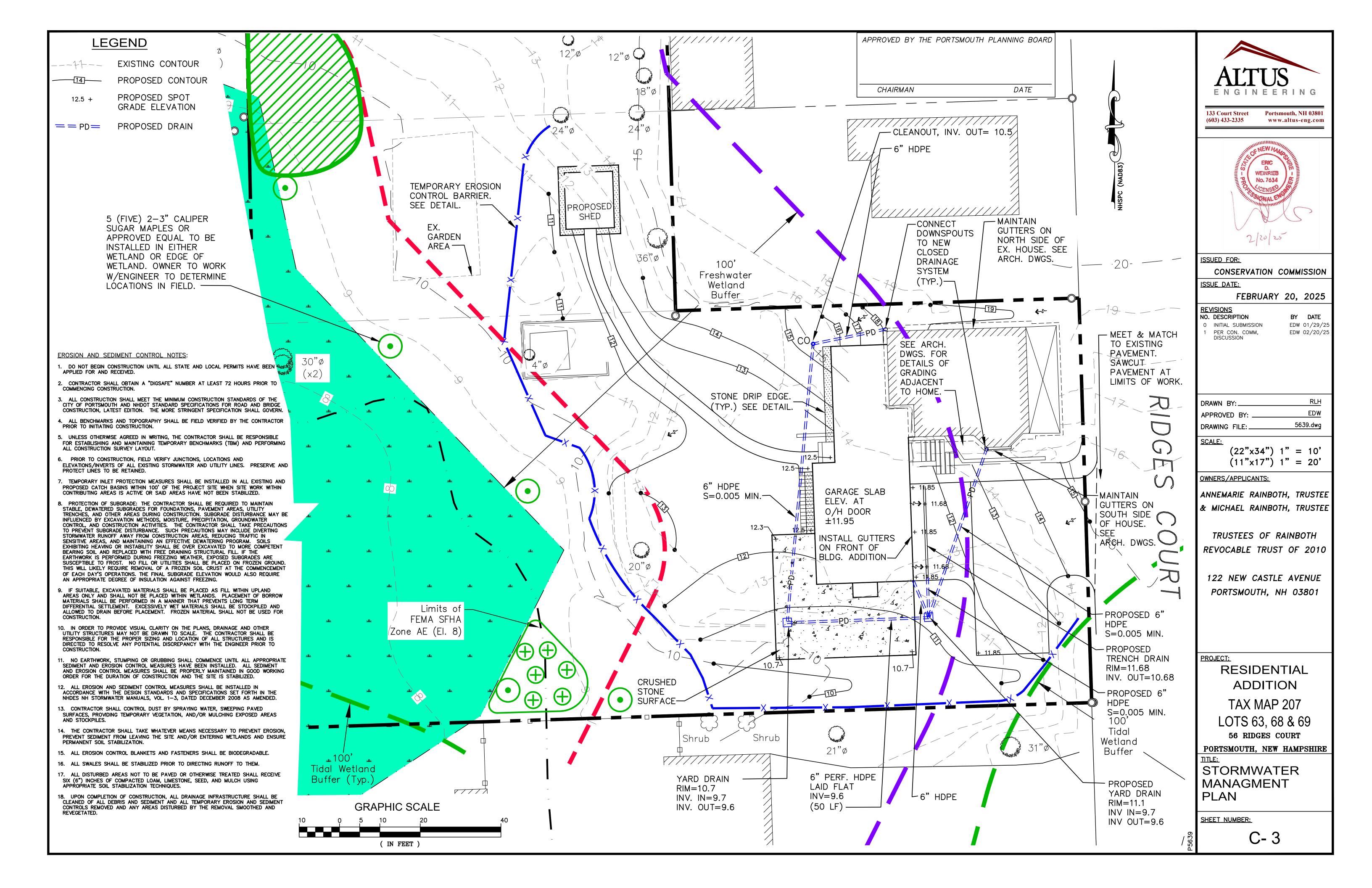


Sheet Index Title	$Sheet \ No.:$	Rev.	$\it Date$
Existing Conditions Plan (by Easterly)	1 OF 1	0	02/22/24
Site Preparation Plan	C-1	0	01/29/25
Site Plan	C-2	1	02/20/25
Grading & Drainage Plan	C-3	1	02/20/25
Detail Sheet	D-1	0	01/29/25
Detail Sheet	D-2	1	02/20/25
Detail Sheet	D-3	1	02/20/25
Proposed Foundation Plan	A-8	0	02/17/25
Elevations	A-16	Ο	02/17/25
Elevations	A-17	0	02/17/25









SEDIMENT AND EROSION CONTROL NOTES

PROJECT NAME AND LOCATION

PROPOSED RESIDENTIAL ADDITION & SITE IMPROVEMENTS

PORTSMOUTH, NEW HAMPSHIRE

TAX MAP 207 LOT 63, 68 & 69

LONGITUDE: -70°45'20" W LATITUDE: 43°04'10" N

OWNER / APPLICANT:

56 RIDGES COURT

ANNEMARIE RAINBOTH, TRUSTEE & MICHAEL RAINBOTH, TRUSTEE

TRUSTEES OF RAINBOTH REVOCABLE TRUST OF 2010

122 NEW CASTLE AVENUE PORTSMOUTH, NH 03801

DESCRIPTION

The project consists of the removal of a garage and the construction of an addition to a single-family residential home along with associated site improvements.

DISTURBED AREA

The total area to be disturbed for the redevelopment improvements is approximately 10,500 S.F. (±0.24 acres)

PROJECT PHASING

The proposed project will be completed in one phase

NAME OF RECEIVING WATER

The site drains overland to an unnamed wetland leading to Little Harbor.

SEQUENCE OF MAJOR ACTIVITIES

- 1. Install temporary erosion control measures including silt fences, stabilized construction entrance and inlet sediment filters as noted on the plan. All temporary erosion control measures shall be maintained in good working condition for the duration of the project.
- 2. Raze existing structures or portions thereof as shown, 3. Strip loam and stockpile.
- 4. Construct site features as shown on plan.
- 5. Rough grade site including placement of borrow materials. 6. Construct drainage structures, culverts, utilities & swales.
- 7. Loam (6" min) and seed all disturbed areas not paved or otherwise stabilized.
- 8. Install permeable & impervious driveway.
- 9. When all construction activity is complete and site is stabilized, remove all temporary erosion control measures and any sediment that has been trapped by these devices.

TEMPORARY EROSION & SEDIMENT CONTROL AND STABILIZATION PRACTICES

All work shall be in accordance with state and local permits. Work shall conform to the practices described in the "New Hampshire Stormwater Manual, Volumes 1 - 3", issued December 2008, as amended. As indicated in the sequence of Major Activities, the silt fences shall be installed prior to commencing any clearing or grading of the site. Structural controls shall be installed concurrently with the applicable activity. Once construction activity ceases permanently in an area, silt fences and any earth/dikes will be removed once permanent measures are established.

During construction, runoff will be diverted around the site with stabilized channels where possible Sheet runoff from the site shall be filtered through hay bale barriers, stone check dams, and silt fences. All storm drain inlets shall be provided with hay bale filters or stone check dams. Stone rip rap shall be provided at the outlets of drain pipes and culverts where shown on the drawings.

Stabilize all ditches, swales, & level spreaders prior to directing flow to them.

Temporary and permanent vegetation and mulching is an integral component of the erosion and sedimentation control plan. All areas shall be inspected and maintained until vegetative cover is established. These control measures are essential to erosion prevention and also reduce costly rework of graded and shaped areas

Temporary vegetation shall be maintained in these areas until permanent seeding is applied. Additionally, erosion and sediment control measures shall be maintained until permanent vegetation is

INSTALLATION, MAINTENANCE AND INSPECTION PROCEDURES FOR TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES

A. GENERAL

These are general inspection and maintenance practices that shall be used to implement the

- The smallest practical portion of the site shall be denuded at one time.
- 2. All control measures shall be inspected at least once each week and following any storm event
- 3. All measures shall be maintained in good working order; if a repair is necessary, it will be initiated within 24 hours. 4. Built-up sediment shall be removed from silt fence or other barriers when it has reached
- one-third the height of the fence or bale, or when "bulges" occur. 5. All diversion dikes shall be inspected and any breaches promptly repaired
- 6. Temporary seeding and planting shall be inspected for bare spots, washouts, and unhealthy
- 7. The owner's authorized engineer shall inspect the site on a periodic basis to review compliance
- 8. An area shall be considered stable if one of the following has occurred:
- a. Base coarse gravels have been installed in areas to be paved;
- b. A minimum of 85% vegetated growth as been established;
- c. A minimum of 3 inches of non-erosive material such as stone of riprap has been installed;
- d. Erosion control blankets have been properly installed.

the National Weather Service in Concord, to have adequate warning of

9. The length of time of exposure of area disturbed during construction shall not exceed 45 days.

B. MULCHING

Mulch shall be used on highly erodible soils, on critically eroding areas, on areas where conservation of moisture will facilitate plant establishment, and where shown on the plans.

- Timing In order for mulch to be effective, it must be in place prior to major storm events. There are two (2) types of standards which shall be used to assure this: a. Apply mulch prior to any storm event. This is applicable when working within 100 feet of wetlands. It will be necessary to closely monitor weather predictions, usually by contacting
- significant storms. b. Required Mulching within a specified time period. The time period can range from 21 to 28 days of inactivity on a area, the length of time varying with site conditions. Professional judgment shall be used to evaluate the interaction of site conditions (soil erodibility, season of year, extent of disturbance, proximity to sensitive resources, etc.) and the potential impact of erosion on adjacent areas to choose an appropriate time restriction.

INSTALLATION, MAINTENANCE AND INSPECTION PROCEDURES FOR

TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES (CON'T)

2. Guidelines for Winter Mulch Application -

pe y or Straw	Rate per 1.000 s.f. 70 to 90 lbs.	Use and Comments Must be dry and free from mold. May be used with plantings.
ood Chips or rk Mulch	460 to 920 lbs.	Used mostly with trees and shrub plantings.
te and Fibrous Itting (Erosion Inket	As per manufacturer Specifications	Used in slope areas, water courses and other Control areas.
ushed Stone 4" to 1-1/2" dia.	Spread more than 1/2" thick	Effective in controlling wind and water erosion.

2" thick (min)

* The organic matter content is between 80 and 100%, dry weight basis. * Particle size by weight is 100% passing a 6"screen and a minimum of 70 %, maximum of 85%, passing a 0.75" screen. * The organic portion needs to be fibrous and elongated. * Large portions of silts, clays or fine sands are not acceptable in the mix. * Soluble salts content is less than 4.0 mmhos/cm.

* The pH should fall between 5.0 and 8.0.

- 3. Maintenance All mulches must be inspected periodically, in particular after rainstorms, to check for rill erosion. If less than 90% of the soil surface is covered by mulch, additional mulch shall be immediately applied.
- C. TEMPORARY GRASS COVER

Erosion Control Mix

1. Seedbed Preparation -

Apply fertilizer at the rate of 600 pounds per acre of 10-10-10. Apply limestone (equivalent to 50 percent calcium plus magnesium oxide) at a rate of three (3) tons per acre.

2. Seeding -

- a. Utilize annual rye grass at a rate of 40 lbs/acre.
- b. Where the soil has been compacted by construction operations, loosen soil to a depth of two (2) inches before applying fertilizer, lime and seed.
- c. Apply seed uniformly by hand, cyclone seeder, or hydroseeder (slurry including seed and fertilizer). Hydroseedings, which include mulch, may be left on soil surface. Seeding rates must be increased 10% when hydroseeding.

3. Maintenance -

Temporary seedings shall be periodically inspected. At a minimum, 95% of the soil surface should be covered by vegetation. If any evidence of erosion or sedimentation is apparent, repairs shall be made and other temporary measures used in the interim (mulch, filter barriers, check dams, etc.).

D. FILTERS

1. Sequence of Installation -

Sediment barriers shall be installed prior to any soil disturbance of the contributing upslope drainage area.

- a. Silt fence barriers shall be inspected immediately after each rainfall and at least daily during prolonged rainfall. They shall be repaired if there are any signs of erosion or sedimentation below them. Any required repairs shall be made immediately. If there are signs of undercutting at the center or the edges, or impounding of large volumes of water, the sediment barriers shall be replaced with a temporary stone check dam.
- b. Should the fabric on a silt fence or filter barrier decompose or become ineffective prior to the end of the expected usable life and the barrier still is necessary, the fabric shall be
- a. Sediment deposits must be removed when deposits reach approximately one-third (1/3) the
- b. Any sediment deposits remaining in place after the silt fence or other barrier is no longer required shall be removed. The area shall be prepared and seeded.
- c. Additional stone may have to be added to the construction entrance, rock barrier and riprap lined swales, etc., periodically to maintain proper function of the erosion control

E. PERMANENT SEEDING -

- 1. Bedding stones larger than $1\frac{1}{2}$ ", trash, roots, and other debris that will interfere with seeding and future maintenance of the area should be removed. Where feasible, the soil should be tilled to a depth of 5" to prepare a seedbed and mix fertilizer into the soil.
- 2. Fertilizer lime and fertilizer should be applied evenly over the area prior to or at the time of seeding and incorporated into the soil. Kinds and amounts of lime and fertilizer should be based on an evaluation of soil tests. When a soil test is not available, the following minimum amounts should be applied:

Agricultural Limestone @ 100 lbs. per 1,000 s.f. 10-20-20 fertilizer @ 12 lbs. per 1,000 s.f.

3. Seed Mixture (recommended):

<u>Type</u> Tall Fescue	Lbs. / Acre 24	<u>Lbs. / 1,000 st</u> 0.55
Creeping Red Fescue	24	0.55
Total	48	1 10

Seed Mixture (For slope embankments):

Grass Seed: Provide fresh, clean, new-crop seed complying with tolerance for purity and germination established by Official Seed Analysts of North America. Provide seed mixture composed of grass species, proportions and minimum percentages of purity, germination, and maximum percentage of weed seed, as specified:

Type	Min. <u>Purity (%)</u>	Min. Germination (%)	Kg./Hectare (Lbs/Acre)
Creeping Red Fescue (c)	96	85	45 (40)
Perennial Rye Grass (a)	98	90	35 (30)
Redtop	95	80	5 (5)
Alsike Clover	97	90(e)	5 (5)
		Tota	90 (80)

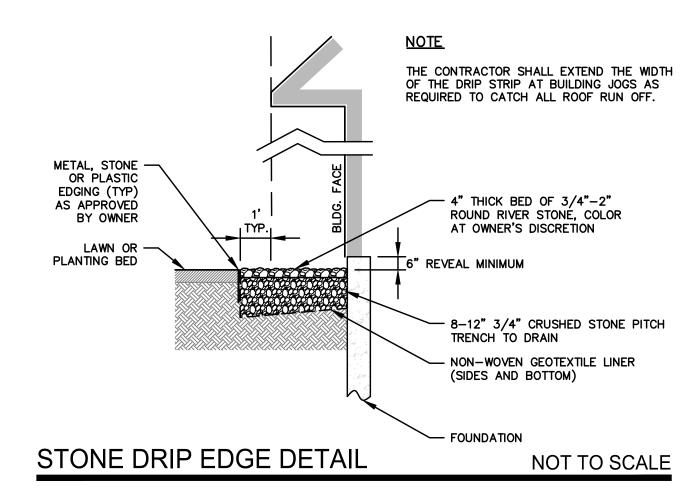
- a. Ryegrass shall be a certified fine—textured variety such as Pennfine, Fiesta, Yorktown, Diplomat, or equal.
- b. Fescue varieties shall include Creeping Red and/or Hard Reliant, Scaldis, Koket, or Jamestown.

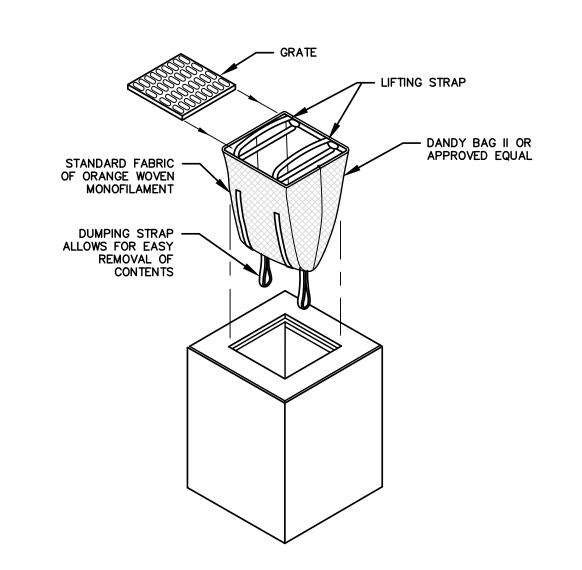
INSTALLATION, MAINTENANCE AND INSPECTION PROCEDURES FOR TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES (CON'T)

4. Sodding - sodding is done where it is desirable to rapidly establish cover on a disturbed area. Sodding an area may be substituted for permanent seeding procedures anywhere on site. Bed preparation, fertilizing, and placement of sod shall be performed according to the S.C.S. Handbook. Sodding is recommended for steep sloped areas, areas immediately adjacent to sensitive water courses, easily erodible soils (fine sand/silt), etc.

WINTER CONSTRUCTION NOTES

- 1. All proposed vegetated areas which do not exhibit a minimum of 85% vegetative growth by October 15th, or which are disturbed after October 15th, shall be stabilized by seeding and installing erosion control blankets on slopes greater than 3:1, and elsewhere seeding and placing 3 to 4 tons of mulch per acre, secured with anchored netting. The installation of erosion control blankets or mulch and netting shall not occur over accumulated snow or on frozen ground and shall be completed in advance of thaw or spring melt events;
- 2. All ditches or swales which do not exhibit a minimum of 85% vegetative growth by October 15th, or which are disturbed after October 15th, shall be stabilized temporarily with stone or erosion control blankets appropriate for the design flow conditions; and
- 3. After November 15th, incomplete road or parking surfaces where work has stopped for the winter season shall be protected with a minimum of 3 inches of crushed gravel per NHDOT Item 304.3.





INSTALLATION AND MAINTENANCE:

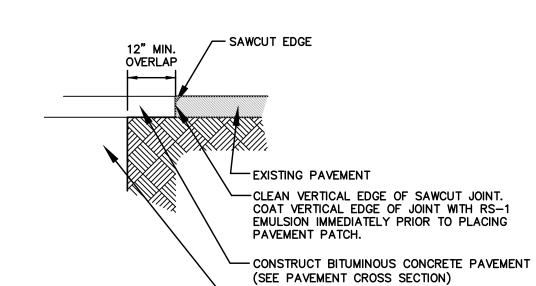
INSTALLATION: REMOVE THE GRATE FROM CATCH BASIN. IF USING OPTIONAL OIL ABSORBENTS; PLACE ABSORBENT PILLOW IN UNIT. STAND GRATE ON END. MOVE THE TOP LIFTING STRAPS OUT OF THE WAY AND PLACE THE GRATE INTO CATCH BASIN INSERT SO THE GRATE IS BELOW THE TOP STRAPS AND ABOVE THE LOWER STRAPS. HOLDING THE LIFTING DEVICES, INSERT THE GRATE INTO THE INLET.

MAINTENANCE: REMOVE ALL ACCUMULATED SEDIMENT AND DEBRIS FROM VICINITY OF THE UNIT AFTER EACH STORM EVENT. AFTER EACH STORM EVENT AND AT REGULAR INTERVALS, LOOK INTO THE CATCH BASIN INSERT. IF THE CONTAINMENT AREA IS MORE THAN 1/3 FULL OF SEDIMENT, THE UNIT MUST BE EMPTIED. TO EMPTY THE UNIT, LIFT THE UNIT OUT OF THE INLET USING THE LIFTING STRAPS AND REMOVE THE GRATE. IF USING OPTIONAL ABSORBENTS; REPLACE ABSORBENT WHEN NEAR SATURATION.

UNACCEPTABLE INLET PROTECTION METHOD:

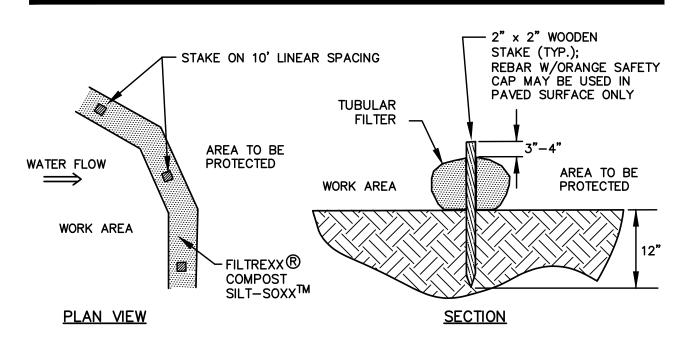
A SIMPLE SHEET OF GEOTEXTILE UNDER THE GRATE IS NOT ACCEPTABLE.

STORM DRAIN INLET PROTECTION NOT TO SCALE



TYPICAL PAVEMENT SAWCUT

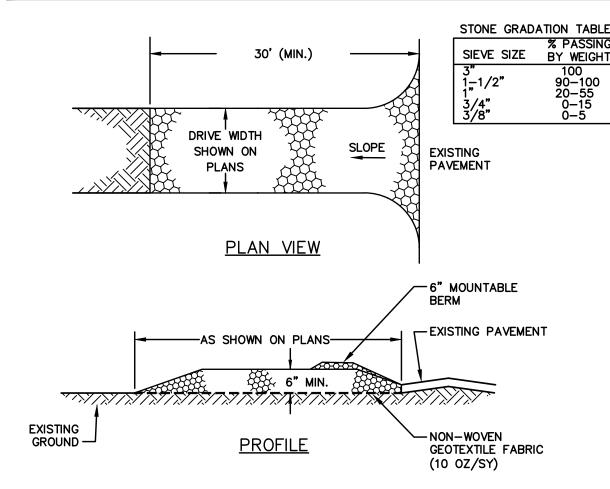
NOT TO SCALE



- TRENCH OR OTHER EXCAVATION PER PLANS

- NOTES:
 1. SILTSOXX OR APPROVED EQUAL SHALL BE USED FOR TUBULAR SEDIMENT BARRIERS.
- 2. ALL MATERIAL TO MEET MANUFACTURER'S SPECIFICATIONS. 3. COMPOST/SOIL/ROCK/SEED FILL MATERIAL SHALL BE ADJUSTED AS NECESSARY TO MEET THE
- REQUIREMENTS OF THE SPECIFIC APPLICATION. 4. ALL SEDIMENT TRAPPED BY BARRIER SHALL BE DISPOSED OF PROPERLY.

TUBULAR SEDIMENT BARRIER DETAIL NOT TO SCALE



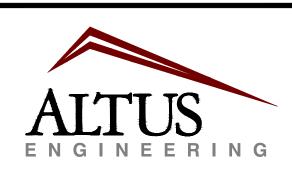
CONSTRUCTION SPECIFICATIONS

- 1. STONE SIZE NHDOT STANDARD STONE SIZE #4 SECTION 703 OF NHDOT STANDARD.
- 2. <u>LENGTH</u> DETAILED ON PLANS (50 FOOT MINIMUM).
- 3. THICKNESS SIX (6) INCHES (MINIMUM).
- 4. WIDTH FULL DRIVE WIDTH UNLESS OTHERWISE SPECIFIED. 5. FILTER FABRIC - MIRAFI 600X OR EQUAL APPROVED BY ENGINEER.
- SURFACE WATER CONTROL ALL SURFACE WATER THAT IS FLOWING TO OR DIVERTED TOWARD THE
- CONSTRUCTION ENTRANCE SHALL BE PIPED BENEATH THE ENTRANCE. IF PIPING IS IMPRACTICAL, A BERM WITH 5:1 SLOPES THAT CAN BE CROSSED BY VEHICLES MAY BE SUBSTITUTED FOR THE PIPE. MAINTENANCE - THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS-OF-WAY. THIS WILL REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL STONE OR ADDITIONAL LENGTH AS CONDITIONS DEMAND AND

REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED,

- DROPPED, WASHED OR TRACKED ONTO PUBLIC RIGHTS-OF-WAY MUST BE REMOVED IMMEDIATELY. WHEELS SHALL BE CLEANED TO REMOVE MUD PRIOR TO ENTRANCE ONTO PUBLIC RIGHTS-OF-WAY. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH STONE WHICH DRAINS INTO AN APPROVED SEDIMENT TRAPPING DEVICE.
- STABILIZED CONSTRUCTION EXITS SHALL BE INSTALLED AT ALL ENTRANCES TO PUBLIC RIGHTS-OF-WAY, AT LOCATIONS SHOWN ON THE PLANS, AND/OR WHERE AS DIRECTED BY THE

STABILIZED CONSTRUCTION EXIT NOT TO SCALE



Portsmouth, NH 03801

www.altus-eng.com

WEINRIEB No. 7634

ISSUED FOR:

NO. DESCRIPTION

133 Court Street

INITIAL SUBMISSION

BY DATE

EDW 01/29/25

ISSUE DATE: JANUARY 29, 2025

<u>REVISIONS</u>

INITIAL SUBMISSION

RLH DRAWN BY:

5639-DETAILS.dwg

APPROVED BY:

DRAWING FILE: ____

(22"x34") NOT TO SCALE (11"x17") NOT TO SCALE

DWNERS/APPLICANTS:

ANNEMARIE RAINBOTH, TRUSTEE & MICHAEL RAINBOTH, TRUSTEE

TRUSTEES OF RAINBOTH REVOCABLE TRUST OF 2010

122 NEW CASTLE AVENUE PORTSMOUTH, NH 03801

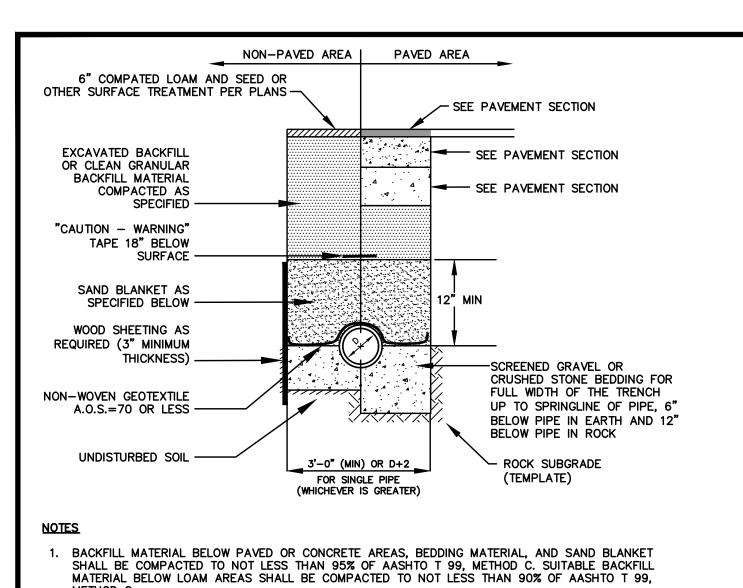
RESIDENTIAL **ADDITION** TAX MAP 207 LOTS 63, 68 & 69 **56 RIDGES COURT**

PORTSMOUTH, NEW HAMPSHIRE

EROSION CONTROL NOTES & DETAILS

SHEET NUMBER:

D-1



2. INSULATE GRAVITY SEWER AND FORCEMAINS WHERE THERE IS LESS THAN 5'-0" OF COVER WITH 2" THICK CLOSED CELL RIGID BOARD INSULATION, 18" ON EACH SIDE OF PIPE.

. MAINTAIN 12" MINIMUM HORIZONTAL SEPARATION AND WIDEN TRENCH ACCORDINGLY IF MULTIPLE PIPES ARE IN TRENCH.

SAND E	BLANKET/BARRIER	SCREENED GRAVEL C	OR CRUSHED STONE BEDDING*
SIEVE SIZE	% FINER BY WEIGHT	SIEVE SIZE	% PASSING BY WEIGHT
1/2" 200	90 - 100 0 - 15	1" 3/4" 3/8" # 4 # 8	100 90 - 100 20 - 55 0 - 10 0 - 5
		* FOUIVALENT TO STAND	ARD STONE SIZE #67 -

SECTION 703 OF NHDOT STANDARD SPECIFICATIONS

PIPES(S) AS

SPECIFIED -

6" PERFORATED HDPE —

N-12 BY ADS OR

APPROVED EQUAL

SCARIFIED SUBGRADE;

FILL AREA: LIGHTLY COMPACTED NHDOT 304.1 SAND

PERVIOUS DECK DETAIL

(PERCOLATION RATE NOT TO EXCEED 4 MIN/SEC)-

DRAINAGE TRENCH

FRAMES AND GRATES SHALL BE DUCTILE IRON PER ASTM

DRAIN BASIN TO BE CUSTOM

DRAINAGE CONNECTION STUB

CONFORM TO ASTM D3212 FOR

DIAMETER AS SPECIFIED AND AS MANUFACTURED BY ADS OR

INSTALL THE DRAIN BASIN PER

CORRUGATED HDPE, N-12HP

INLINE DRAIN TO BE PVC.

THE CONTRACTOR SHALL

THE MANUFACTURER'S

FOR INSTALLATION IN

AREAS ONLY.

YARD DRAIN

SEE GRADING PLAN FOR -

ALL FINISHED GRADES

ELEV. ±10.7'

ELEV. 9.6'

ELEV. 8.6'

RECOMMENDATIONS AND AS SHOWN ON THE DRAWINGS.

PEDESTRIAN AND LANDSCAPE

JOINT TIGHTNESS SHALL

MANUFACTURED ACCORDING TO

A536 GRADE 70-50-05.

PLAN AND DETAILS.

AND PVC SEWER.

APPROVED EQUAL.

ANY DAMAGE TO THE CROWN, TRUNK OR ROOT SYSTEM OF ANY TREE RETAINED ON SITE SHOULD BE REPAIRED IMMEDIATELY. CONSULT A FORESTER OR TREE SPECIALIST FOR MORE SERIOUS DAMAGE TRUNK ARMORING CONTRACTOR TO USE OPTION A OR B WHERE OPTION B SUITABLE AND/OR AS DIRECTED BY THE ENGINEER.

- BURLAP AGAINST TREE

- DRIP LINE OR MIN. PLASTIC FENCING AROUND

F SOIL BECOMES COMPATCED OVER THE ROOT ZONE OF ANY TREE, THE GROUND SHOULD BE AERATED BY

PUNCHING SMALL HOLES IN IT WITH SUITABLE

AERATING EQUIPMENT.

OPTION A TREE PROTECTION DETAILS

— DECK BY OTHERS

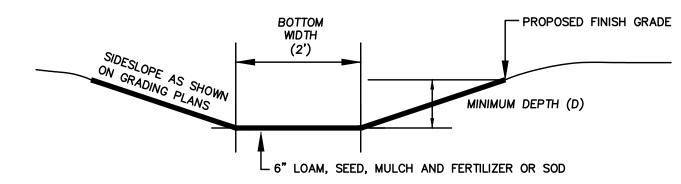
- 3/4" - 1-1/2" CLEAN

CRUSHED ANGULAR STONE

NON-WOVEN FILTER FABRIC ALL AROUND

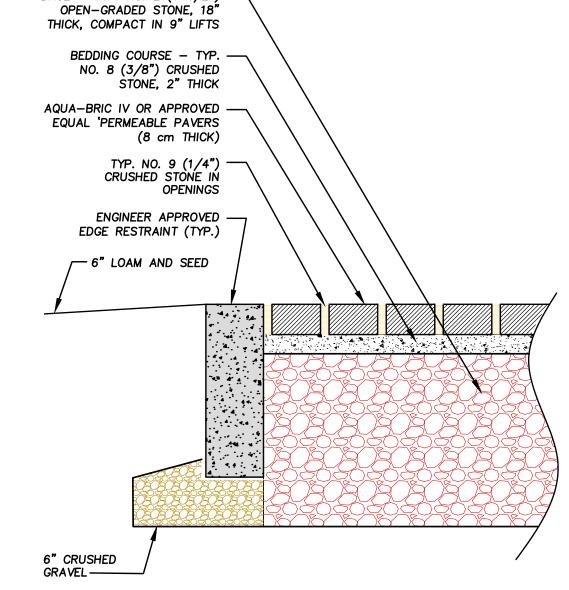
NOT TO SCALE

NOT TO SCALE



NOTES

- 1. THE FOUNDATION AREA OF THE WATERWAY SHALL BE CLEARED AND GRUBBED OF ALL TREES, BRUSH, STUMPS, AND OTHER OBJECTIONABLE MATERIAL. MATERIALS REMOVED SHALL BE DISPOSED OF SO THEY WILL NOT INTERFERE WITH THE CONSTRUCTION OR PROPER FUNCTIONING OF THE WATERWAY. 2. THE WATERWAY SHALL BE EXCAVATED OR SHAPED TO LINE, GRADE AND CROSS SECTION AS REQUIRED TO MEET THE DESIGN CRITERIA. THE WATERWAY SHALL BE FREE OF IRREGULARITIES WHICH
- WILL IMPEDE NORMAL FLOW. 3. EARTH FILLS REQUIRED TO MEET SUBGRADE REQUIREMENTS BECAUSE OF OVER EXCAVATION OR TOPOGRAPHY SHALL BE COMPACTED TO THE SAME DENSITY AS THE SURROUNDING SOIL TO PREVENT UNEQUAL SETTLEMENT THAT COULD CAUSE DAMAGE TO THE COMPLETED WATERWAY. EARTH REMOVED AND NOT NEEDED IN CONSTRUCTION SHALL BE SPREAD OR DISPOSED OF SO IT WILL NOT
- INTERFERE WITH THE FUNCTIONING OF THE WATERWAY. 4. CONSTRUCTION OPERATIONS SHALL BE CARRIED OUT IN SUCH A MANNER AS TO MINIMIZE EROSION AND AIR AND WATER POLLUTION. ALL APPROPRIATE STATE AND LOCAL LAWS AND REGULATIONS
- SHALL BE COMPLIED WITH FOR INSTALLATION. 5. VEGETATION SHALL BE ESTABLISHED IN THE SWALE PRIOR TO ALLOWING STORMWATER RUNOFF TO FLOW THROUGH THE SWALE.
- MAINTENANCE OF THE VEGETATION IN THE GRASSED WATERWAY IS EXTREMELY IMPORTANT IN ORDER TO PREVENT RILLING, EROSION, AND FAILURE OF THE WATERWAY. MOWING SHOULD BE DONE FREQUENTLY ENOUGH TO CONTROL ENCROACHMENT OF WEEDS AND WOODY VEGETATION AND TO KEEP THE GRASSES IN A VIGOROUS CONDITION. THE VEGETATION SHOULD NOT BE MOWED TOO CLOSELY SO AS TO REDUCE THE EROSION RESISTANCE IN THE WATERWAY.
- 7. THE WATERWAY SHOULD BE INSPECTED PERIODICALLY AND AFTER EVERY MAJOR STORM TO DETERMINE THE CONDITION OF THE WATERWAY. RILLS AND DAMAGED AREAS SHOULD BE PROMPTLY REPAIRED AND REVEGETATED AS NECESSARY TO PREVENT FURTHER DETERIORATION.
- 8. PERIODIC APPLICATIONS OF LIME AND FERTILIZER MAY BE NEEDED TO MAINTAIN VIGOROUS GROWTH



BASE - TYP. NO. 2 (1-1/2")

SIEVE SIZE	PERCENT PASSING		
	No. 9 (1/4")	No. 8 (3/8")	No. 2 (1 1/2")
3 in	-	_	100
2 1/2 in	_	_	90 – 100
2 in	_	_	35 - 70
1 1/2 in	_	_	0 - 15
3/4 in	_	_	0 - 5
1/2 in	100	100	_
3/8 in	90 - 100	85 – 100	_
No. 4	20 - 55	10 - 30	_
No. 8	5 - 30	0 - 10	_
No. 16	0 - 10	0 - 5	_
No. 50	0 - 5	-	_

PERMEABLE PAVERS DETAIL

NOT TO SCALE

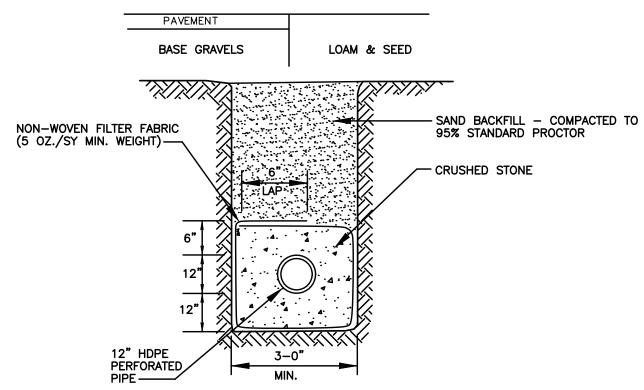
GRASSED SWALE

NOT TO SCALE

PEDESTRIAN GRATE (MAY BE SQUARE, 15" MIN) - ADS DRAIN BASIN OR EQUAL, SIZE BASED ON PIPE CONFIGURATION (15" MIN.) - OUTLET PIPE AS SPECIFIED - 3/4" CRUSHED STONE BEDDING - COMPACTED NATIVE SUBGRADE NOT TO SCALE

— 2" PEA STONE

NOT TO SCALE





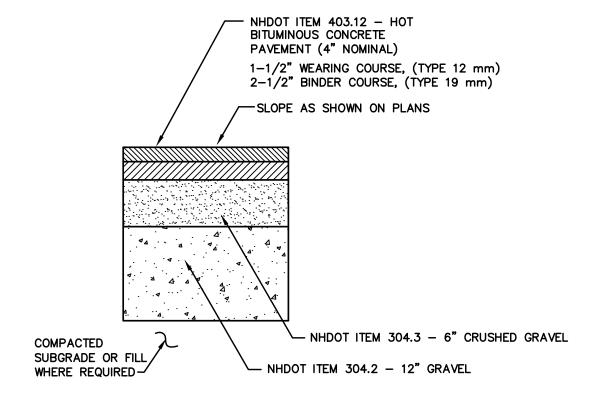
- PIER BY OTHERS

- LOAM AND SEED

3/4" - 1-1/2"

CLEAN CRUSHED

ANGULAR STONE

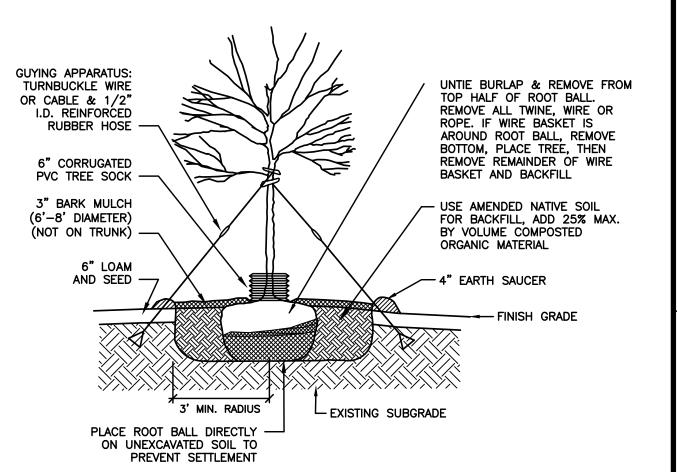


NOTES

- PROJECT GEOTECHNICAL REPORT MAY REQUIRE A DIFFERENT PAVEMENT CROSS SECTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR READING AND FOLLOWING ALL RECOMMENDATIONS IN THE GEOTECHNICAL REPORT. IN THE EVENT THAT THE REPORT AND CIVIL PLANS DIFFER, THE MORE STRINGENT SPECIFICATION SHALL APPLY.
- 2. ALL EXISTING FILL, BURIED ORGANIC MATTER, CLAY, LOAM, MUCK, AND/OR OTHER QUESTIONABLE MATERIAL SHALL BE REMOVED FROM BELOW ALL PAVEMENT, SHOULDERS AND UNDERGROUND PIPING/UTILITIES TO DEPTHS RECOMMENDED IN GEOTECHNICAL REPORT
- 3. SUBGRADE SHALL BE PROOFROLLED A MINIMUM OF 6 PASSES WITH A 10-TON VIBRATORY COMPACTOR OPERATING AT PEAK RATED FREQUENCY OR BY MEANS APPROVED BY THE ENGINEER.
- 4. FILL BELOW PAVEMENT GRADES SHALL BE GRANULAR BORROW COMPACTED PER NHDOT REQUIREMENTS.
- 5. SITEWORK CONTRACTOR SHALL COORDINATE GEOTECHNICAL ENGINEERING INSPECTIONS WITH THE CONSTRUCTION MANAGER PRIOR TO PLACING GRAVELS.
- 6. TACK COAT SHALL BE APPLIED BETWEEN SUCCESSIVE LIFTS OF ASPHALT.
- 7. THE BITUMINOUS PAVEMENT SHALL BE COMPACTED TO 92 TO 97 PERCENT OF ITS THEORETICAL MAXIMUM DENSITY AS DETERMINED BY ASTM D-2041. THE BASE AND SUBBASE MATERIALS SHOULD BE COMPACTED TO AT LEAST 95 PERCENT OF THEIR MAXIMUM DRY DENSITIES AS DETERMINED BY

PAVEMENT CROSS SECTION

NOT TO SCALE

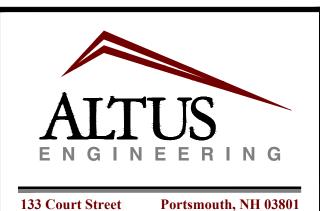


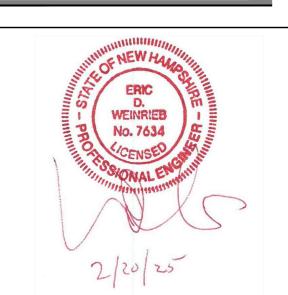
NOTES:

- PLANT TREE SUCH THAT TOP OF ROOT BALL IS FLUSH WITH GRADE (1" 2" HIGHER IN SLOW DRAINING SOIL). TRUNK FLARE MUST BE VISIBLE AT THE TOP OF THE ROOT BALL.
- 2. THREE FLAGGED GUY WIRES TO BE EQUALLY SPACED ABOUT TREE. WOODEN STAKES (24" LENGTH) MAY BE SUBSTITUTED FOR METAL ANCHORS. EITHER OPTION SHALL BE DRIVEN OUTSIDE THE ROOT BALL, PREFERABLY IN UNEXCAVATED SOIL AND REMOVED AT THE END OF THE FIRST GROWING SEASON OR WHEN TREE IS STABILIZED.
- COORDINATE PRUNING WITH LANDSCAPE ARCHITECT WHEN POSSIBLE. DO NOT HEAVILY PRUNE THE TREE AT PLANTING. DO NOT REMOVE THE TERMINAL BUDS OF BRANCHES THAT EXTEND TO THE EDGE OF THE CROWN. PRUNING OF DEAD OR BROKEN BRANCHES OR CO-DOMINANT LEADERS IS PERMITTED.

DECIDUOUS TREE PLANTING

NOT TO SCALE





www.altus-eng.com

ISSUED FOR:

(603) 433-2335

CONSERVATION COMMISSION

<u>ISSUE DATE:</u>

FEBRUARY 20, 2025

BY DATE

EDW 01/29/2

EDW 02/20/25

<u>REVISIONS</u>

NO. DESCRIPTION 0 INITIAL SUBMISSION PER CON. COMM. DISCUSSION

DRAWN BY: APPROVED BY: 5639-DETAILS.dwg DRAWING FILE: ___

(22"x34") NOT TO SCALE (11"x17") NOT TO SCALE

DWNERS/APPLICANTS:

ANNEMARIE RAINBOTH, TRUSTEE & MICHAEL RAINBOTH, TRUSTEE

TRUSTEES OF RAINBOTH REVOCABLE TRUST OF 2010

122 NEW CASTLE AVENUE PORTSMOUTH, NH 03801

PROJECT:

RESIDENTIAL **ADDITION** TAX MAP 207 **LOT 63** 56 RIDGES COURT

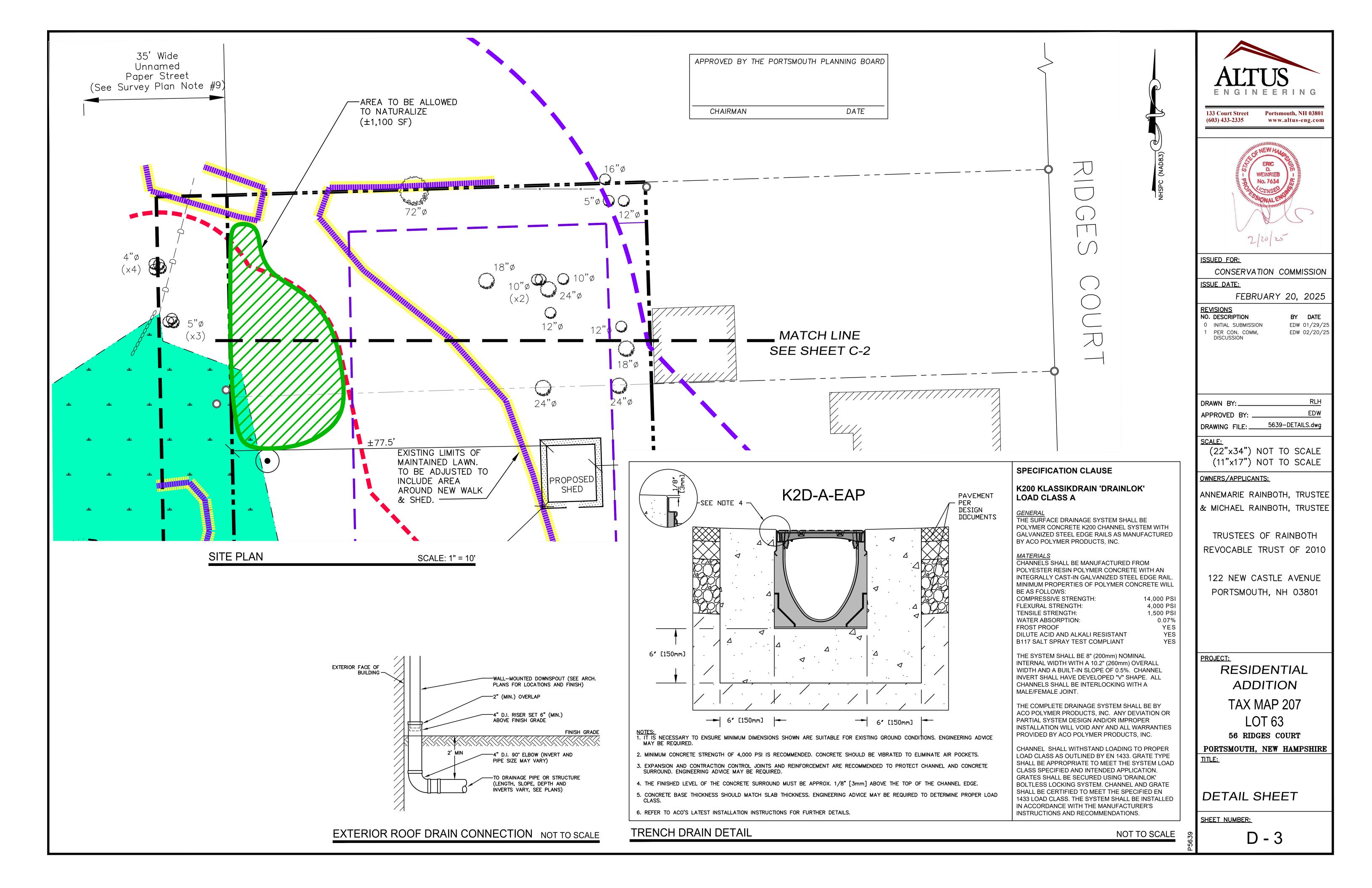
PORTSMOUTH, NEW HAMPSHIRE

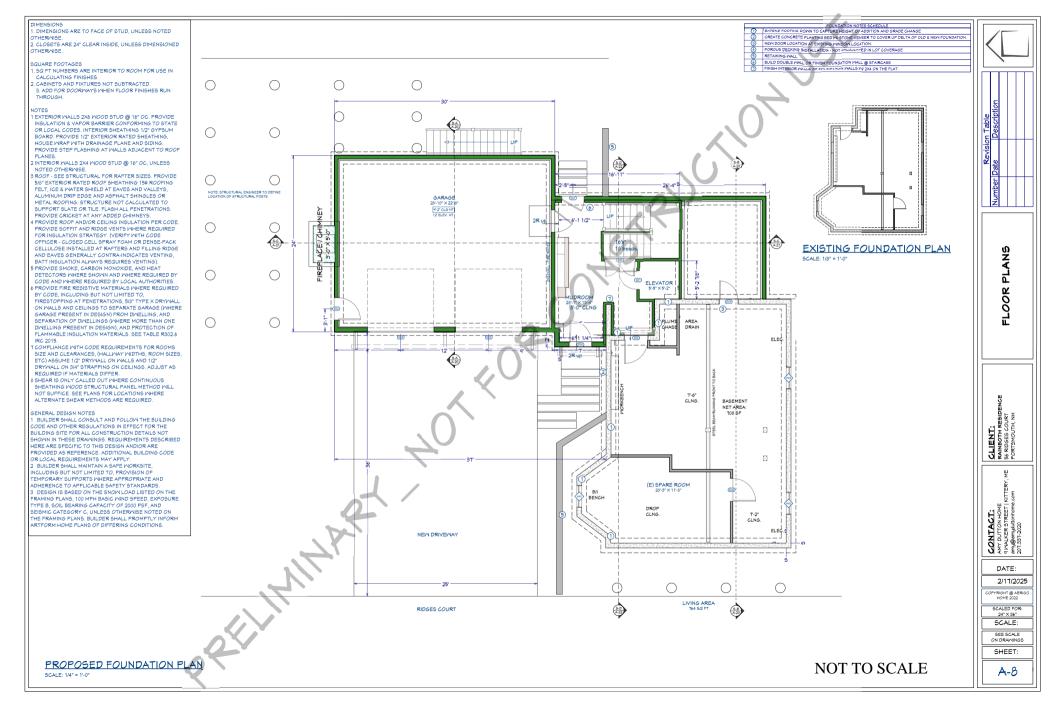
DETAIL SHEET

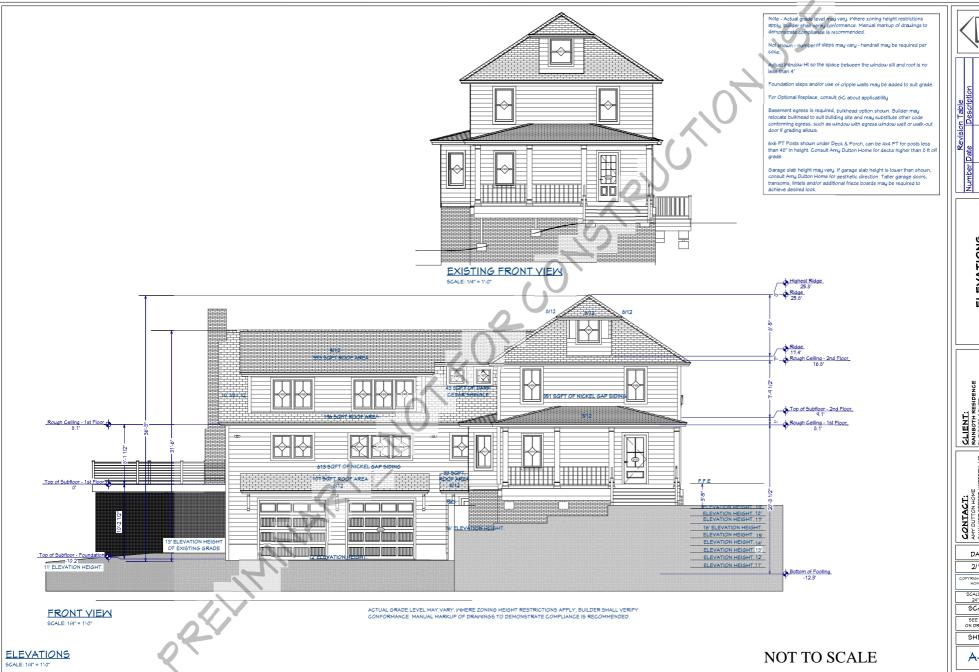
SHEET NUMBER:

D - 2









ELEVATIONS

CLIENT:
RAINBOTH RESIDENCE
56 RIDGES COURT
PORTSMOUTH, NH

CONTACT:
AMY DUTTON HOME
9 WALER STREET | KI
8my@smyduttonhome.o.
201.331-2020

DATE:

2/17/2025

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SCALED FOR: 24" X 36" SCALE:

SEE SCALE ON DRAWINGS

A-16

